WEST ALLISON ROAD CORRIDOR STUDY FINAL REPORT

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Volume 1

Prepared by:

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Cheyenne Metropolitan Planning Office



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STUDY CORRIDOR LOCATION

The West Allison Road Corridor, for purposes of this study, is located in the southwest region of the City of Cheyenne, and runs between US Highway 85 (South Greeley Highway) to the east and South Parsley Boulevard to the west. This section of West Allison Road is approximately 9,000 feet or 1.7 miles in length.

Figure 1. Study Area Location Map



Courtesy of MapQuest

CORRIDOR STUDY BACKGROUND

The West Allison Road Corridor Study was prepared under the direction of a steering committee consisting of some of the following:

Tom Mason - Cheyenne Metropolitan Planning Organization

Nancy Olson - Cheyenne Metropolitan Planning Organization

Doug Vetter - City of Cheyenne Engineering

Sam Berta – City of Cheyenne

Bob Nelson – City of Cheyenne

Tom Bonds - FHWA

Don Beard - Laramie County Public Works Director

Gary Kranse - Laramie County Planning Director

Matt Ashby - City of Cheyenne Urban Planning James Sims - Cheyenne Metropolitan Planning Organization Dennis Auker – Laramie County School District #1

The purpose of this corridor study as designated in the Request for Proposal for the Corridor Study, was to evaluate transportation needs for West Allison Road between South Greeley Highway and Parsley Boulevard. The portion between Walterhscheid Boulevard and Snyder Avenue was excluded from the study because it had recently been upgraded as part of the South High School Project. The common needs determined by the steering committee included transportation demand, safety, legislative direction, urban transportation plan consistency, modal interrelationships, system linkage, and the condition of the existing roadway facility.

This study included the following tasks:

- 1. Public participation process (open house meetings)
- 2. Existing conditions analysis and preparation of Baseline Report
- 3. Drainage Basin Study
- 4. Data collection and conceptual design process
- 5. Conceptual Plan and Final Report Preparation including recommendations

This final report summarizes the tasks above and provides a recommendation for improvements along the West Allison Road corridor.

Existing Pavement Conditions

The existing pavement condition of West Allison Road is very poor between Walterscheid Boulevard and South Greeley Highway. The asphalt roadway contains numerous potholes and both longitudinal and transverse cracking. The edge of the pavement along the section of West Allison Road between South Greeley Highway and Walterscheid Boulevard is breaking apart. These details emphasize the need for improvements as described in the recommendations section of this report.

West Allison Road has recently been reconstructed from Snyder Avenue to Walterscheid Boulevard with the construction of South High School. In addition to the improvements on West Allison Road, a new section of Cribbon Avenue was constructed. This new section connects West Allison Road to Pinto Lane

Existing Traffic Conditions

The West Allison Road right-of-way width varies from 50 feet to 80 feet wide. The roadway is classified by the Plan Cheyenne Transportation Master Plan as a collector street. The corridor consists of the following right-of-way segments:

- South Greeley Highway to Walterscheid Boulevard 50 feet
- Walterscheid Boulevard to Cribbon Avenue 90 feet
- Cribbon Avenue to Arp Avenu 65 feet

This study corridor begins with a signalized intersection at South Greeley Highway, has a signalized intersection at Walterscheid Boulevard, a north leg stop controlled T intersection at Desmet Drive, a roundabout at Snyder Avenue, and a four way stop controlled intersection at Cribbon Avenue. There are multiple residential driveways between South Greeley Highway and Walterscheid Boulevard and Cribbon Street and Arp Avenue as well as driveways for Johnson Junior High School and South High School.

West Allison Road does not connect directly to Parsley Boulevard, but connects via Cribbon Avenue and Jefferson Road. West Jefferson Road is a local residential road that runs directly in front of Goins Elementary School. The Cribbon Avenue connection runs along the westerly edge of the South High School property and connects to Pinto Lane. Pinto Lane is designated as a collector street and has a 70-foot right-of-way. Additional connections are West Allison Road to Ahrens Avenue to either West Jefferson Road to the north or Sundance Lane to the south. Both routes connect to Parsley Boulevard to the west. All of these connections have been considered as part of this study, which will reduce traffic in front of the elementary school as well as congestion in general on the local roads within the residential areas. The connections that have been evaluated include Cribbon Avenue to Pinto Lane and other alignments to reduce traffic congestion and provide better vehicular circulation along the west side of the corridor.

Level of Service Analysis

All intersections along the corridor currently operate at a LOS C or better during the am, mid day and pm peak hours, which is an acceptable delay time. The estimated future traffic volumes included South High School being at full capacity by 2014. All intersections along the corridor with the exception of Greeley Highway and Walterscheid Boulevard will continue to operate at a LOS C or better during the peak hours. Known development projects along the corridor were taken into account when evaluating the growth rate for the future level of service. A summary of LOS, calculated in the Baseline Corridor Study, at each intersection along the corridor has been provided in Table 1.

Table 1. Peak Hour LOS Summary for Study Intersections (including South High traffic)

Intersection	Time Period	Delay (sec / vehicle)	Direction of Travel	LOS
West Allison Road	7-8 AM	21.6 / 29.2	West / East	С
& Greeley Hwy		-	North / South	-
	3-4 PM	21.2 / 22.9	West / East	С
		-	North / South	-
	5-6 PM	21.4 / 38.9	West / East	C/D
		-	North / South	-
West Allison Road	7-8 AM	40.7 / 124.8	West / East	E/F
& Walterscheid Blvd.		8.2 / 7.4	North / South	Α
	3-4 PM	15.2 / 43.3	West / East	C/E
		7.6 / 7.4	North / South	Α
	5-6 PM	14.5 / 14.2	West / East	В
		7.7 / 7.5	North / South	Α
West Allison Road	7-8 AM	15.6	South	С
& Desmet Avenue		8.9	East	Α
	3-4 PM	14.4	South	В
		7.7	East	Α
	5-6 PM	9.8	South	Α
		7.5	East	Α
West Allison Road	7-8 AM	7.71 / 9.19	North / South	Α
& Cribbon Avenue	. 07	7.98 / 8.08	West / East	A
<u> </u>	3-4 PM	7.45 / 7.87	North / South	A
	<u> </u>	7.40 / 7.80	West / East	A
	5-6 PM	7.24 / 8.10	North / South	A
	00110	7.22 / 7.81	West / East	A
Jefferson Road	7-8 AM	9.0	East	A
& Cribbon Avenue	7-0 AIVI	7.4	North	A
& Chibbon Avenue	3-4 PM	9.2		A
	3-4 PIVI		East	
	F C DM	7.4	North	A
	5-6 PM	8.9	East	A
		7.3	North	A
Jefferson Road	7-8 AM	10.2 / 11.0	North / South	<u>B</u>
& Parsley Blvd.		7.2	West	Α
	3-4 PM	11.6 / 13.0	North / South	В
		7.2	West	Α
	5-6 PM	10.0 / 10.5	North / South	A/B
		7.2	West	A
Pinto Lane	7-8 AM	9.6 / 10.1	North / South	A/B
& Parsley Blvd.		7.2 / 7.2	East / West	A
	3-4 PM	10.5 / 11.3	North / South	В
		7.3 / 7.3	East / West	Α
	5-6 PM	11.3 / 11.4	North / South	В
		7.5 / 7.2	East / West	Α
Parsley Blvd.	7-8 AM	9.7 / 9.8	North / South	Α
& Sundance Lane		7.2	West	Α
	3-4 PM	9.9 / 10.6	North / South	A/B
		7.2	West	Α
	5-6 PM	10.2 / 10.3	North / South	В
	<u> </u>	7.2	West	A

Based on the above data and definition of LOS from the Highway Capacity Manual (A-F), the intersections studied, with the exception of Walterscheid Boulevard and South Greeley Highway, will operate at a LOS C or better during the a.m., mid day, and p.m. peak hours once South High School is operating at full capacity.

Vehicle Accident Analysis

From 2005 to 2008 there were a total of 25 vehicle accidents along the corridor. During the four year timeframe, approximately 80 percent of the accidents along the corridor occurred at intersections. The intersection with the most accidents during the four-year period was West Allison Road and South Greeley Highway, with 13 accidents. This intersection also has the highest volume of traffic compared to any other intersection along the corridor. Approximately 20 percent of the accidents occurred within mid-block sections of the corridor. This data helps emphasizes the importance of creating safe intersections. The number of accidents for each intersection along the West Allison Road corridor are shown below in Figure 2.

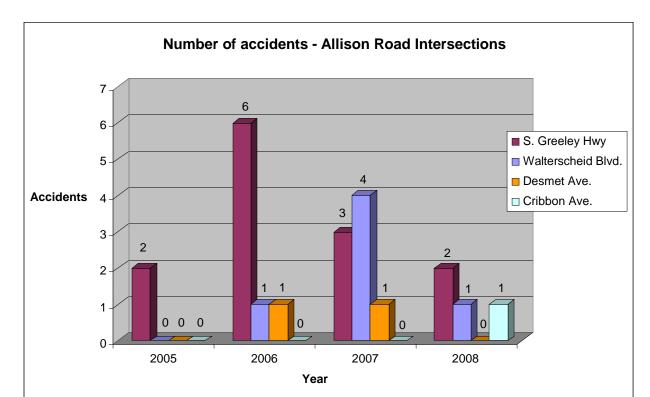


Figure 2: Traffic Crashes per Intersection along West Allison Road

Anticipated Daily Traffic Growth

The forecasted daily 2030 traffic volumes for the West Allison Road Corridor, were developed from Cheyenne MPO's regional travel demand model. This model incorporates measured volumes reported by WyDOT from the previous eight years. The daily traffic volumes on West Allison Road in the study corridor are expected to increase by approximately 2% per year over this 20+ year period. Based on projected WyDOT volumes. Figure 3 shows a graph of the anticipated traffic volumes over a 30 year period. As peak hour congestion occurs, the increase in traffic will result in a longer peak period and more trips occurring at other times during the day. The peak hour traffic volumes at the analyzed intersections for existing and existing with the proposed South High School are shown in Figures 4 and 5.

Figure 3. Projected Average Annual Daily Traffic Volumes (AADT) for West Allison Road with South High

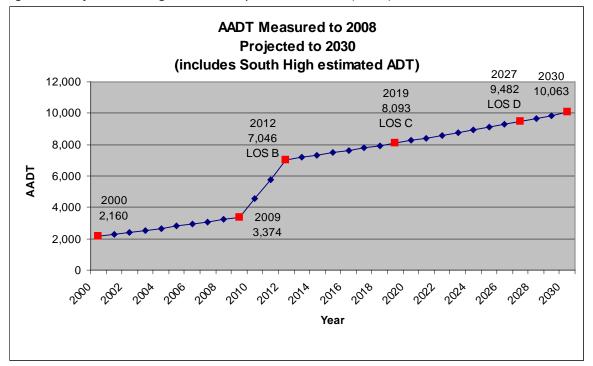
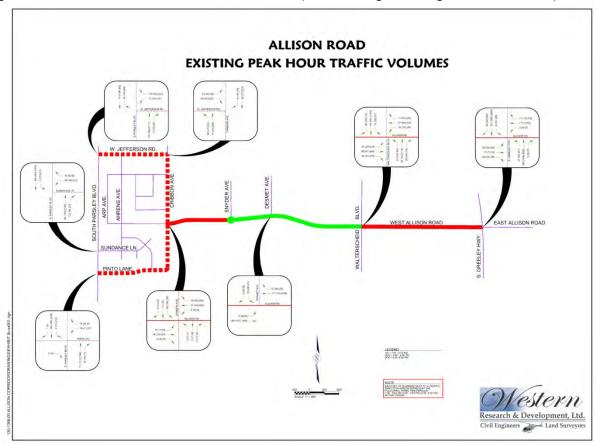
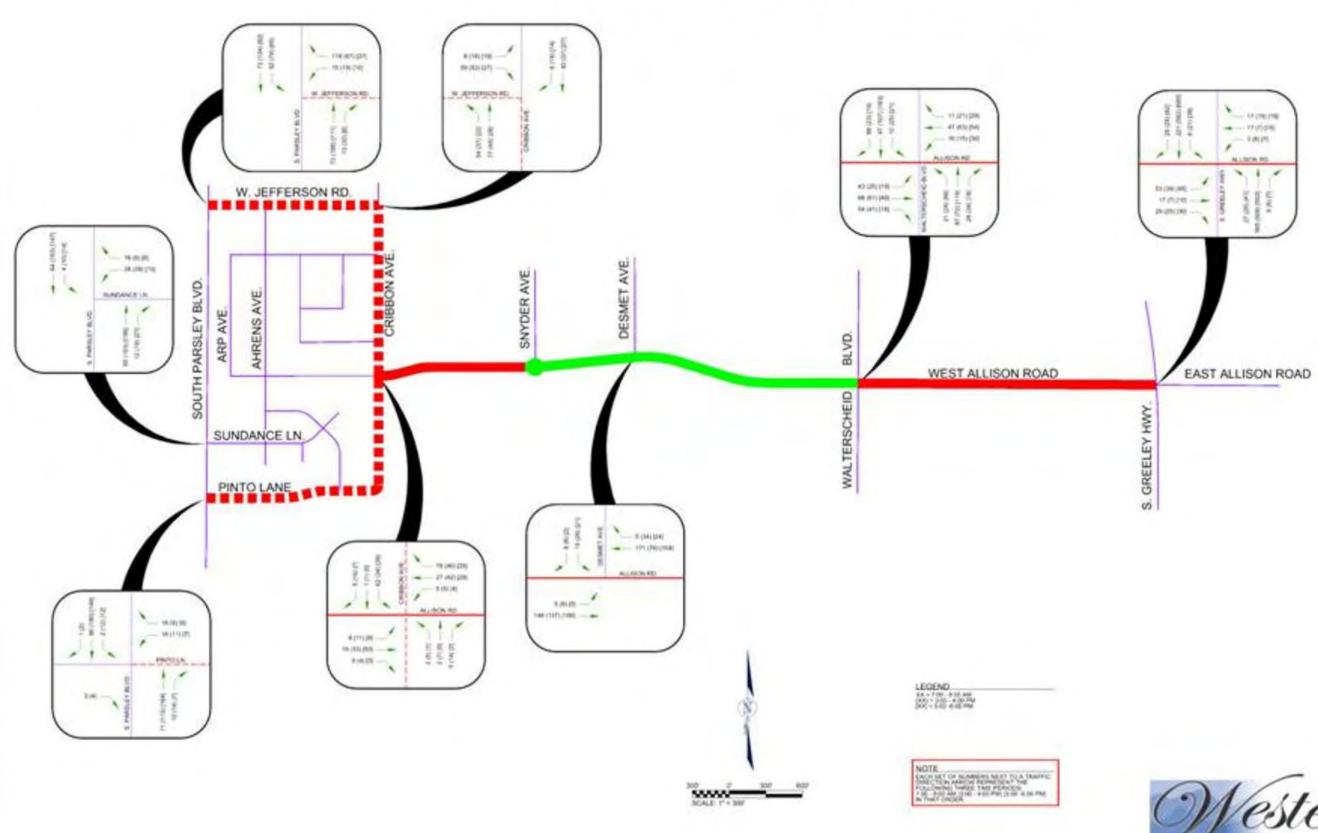


Figure 4: Peak Hour Traffic Volumes at Intersections (not including South High estimated traffic)



ALLISON ROAD EXISTING PEAK HOUR TRAFFIC VOLUMES



Research & Development, Ltd.
Civil Engineers Land Surveyors

ALLISON CORREDORDRAWINGS/EXHBIT 80-ex001.6gm

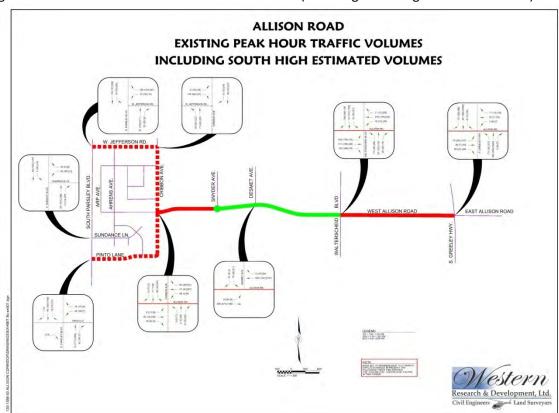
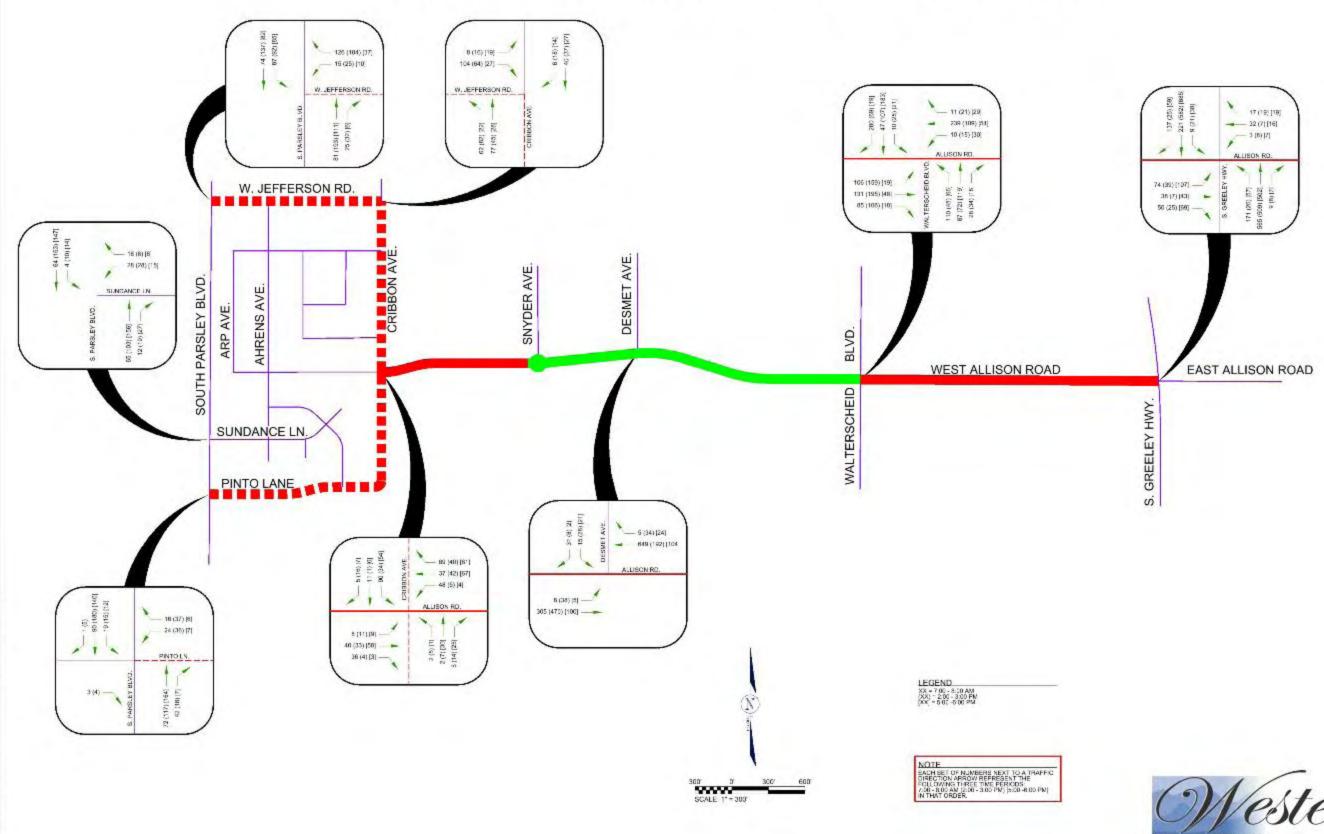


Figure 5: Peak Hour Traffic Volumes at Intersections (including South High estimated traffic)

An additional, potentially larger cause of daily traffic increase on West Allison Road in future years is new development. With the large amount of undeveloped and sparsely developed land along the corridor, it is expected that additional residential and commercial development will occur. The undeveloped land is approximately one square mile north and south of West Allison Road between South Greeley Highway and Walterscheid Boulevard. As this area continues to grow and the traffic volumes continue to increase, it is possible that the increased congestion will eventually require additional road improvements including road widening. The current zoning and future land use is shown in Figures 6 and 7.

ALLISON ROAD EXISTING PEAK HOUR TRAFFIC VOLUMES INCLUDING SOUTH HIGH ESTIMATED VOLUMES



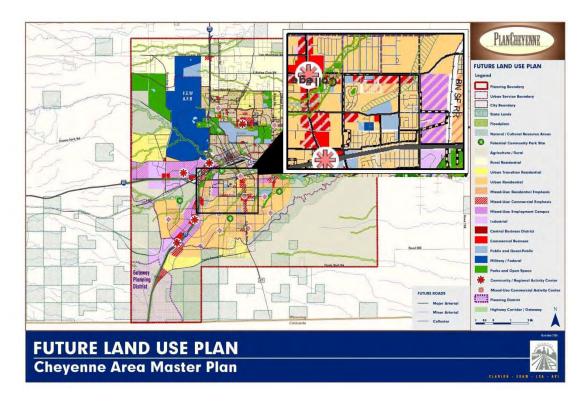
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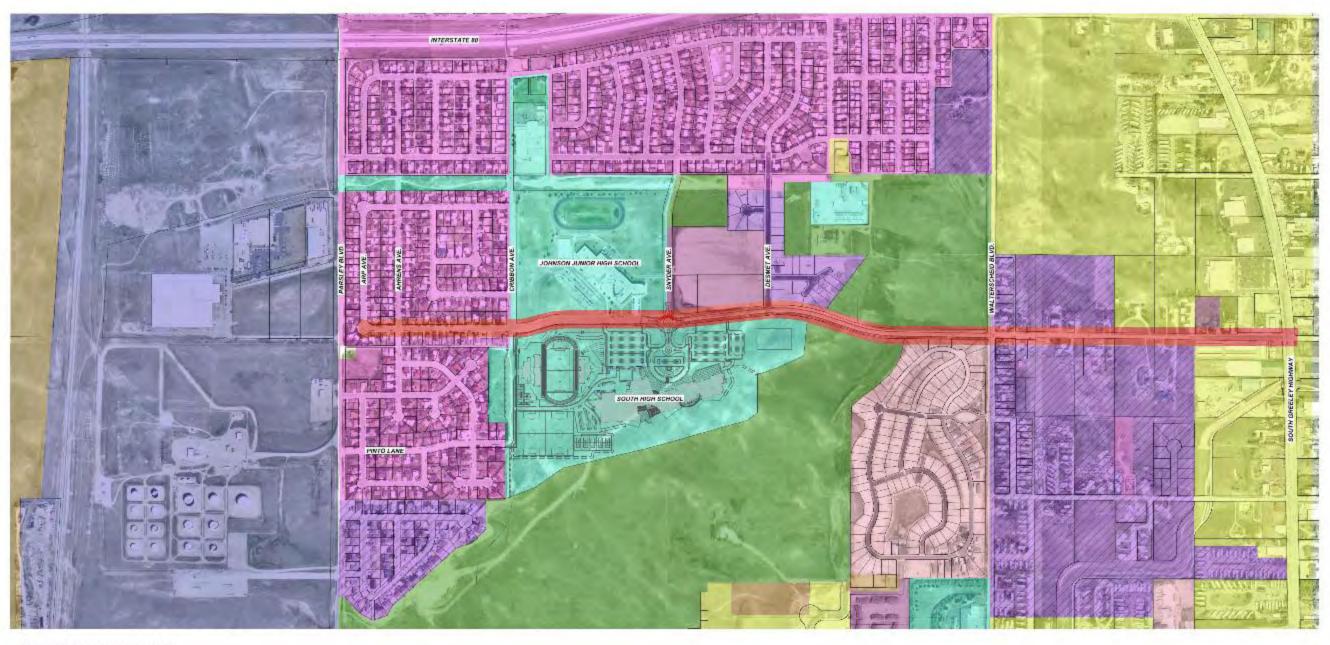
Research & Development, Ltd.
Civil Engineers Land Surveyors

Figure 6. West Allison Road Corridor - Current Zoning Map



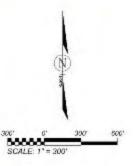
Figure 7. Cheyenne Area Master Plan – Future Land Use Map





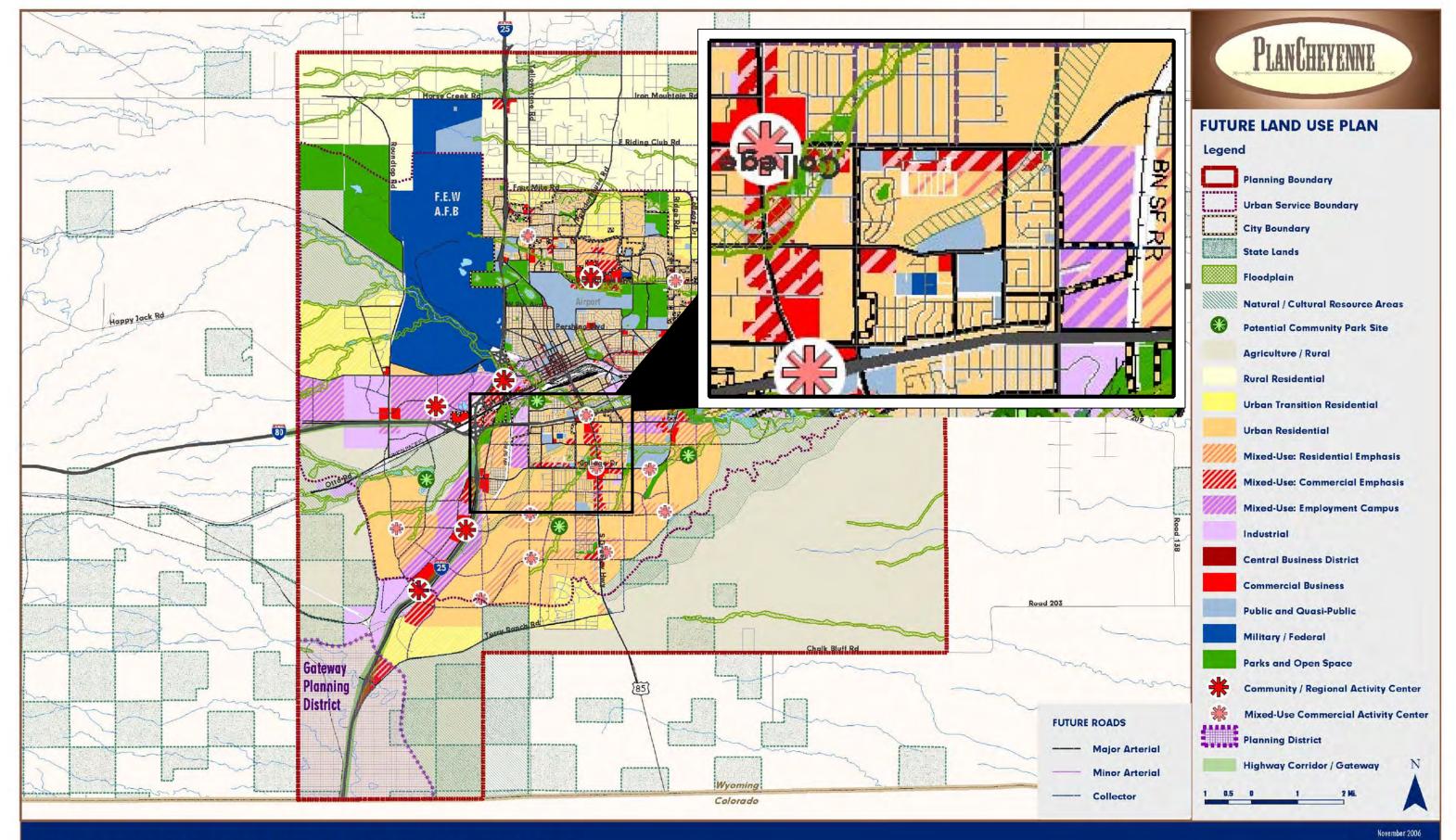
ZONING LEGEND

- MEDIUM DENSITY RESIDENTIAL DEVELOPING (MR-2) MEDIUM DENSITY RESIDENTIAL ESTABLISHED (MR-1)
- COMMUNITY BUSINESS (CB)
- MIXED USE BUSINESS (MUB)
- HIGH DENSITY RESIDENTIAL DEVELOPING (HR-2) PLANNED UNIT DEVELOPMENT (PUD)
- **PUBLIC**
- AGRICULTURAL AND RURAL RESIDENTIAL (A-2)
- LIGHT INDUSTRIAL
- ALLISON ROAD CORRIDOR



ALLISON ROAD EXISTING ZONING CONDITIONS





FUTURE LAND USE PLAN

Cheyenne Area Master Plan



Current Conditions

The study area for West Allison Road is approximately 1.7 miles in length from South Greeley Highway to Parsley Boulevard. West Allison Road serves as a collector street and is also the primary route used by the local residents to gain access to and from their homes. West Allison Road is also the main route to the Laramie County School District #1 school campus which includes Goins Elementary School, Johnson Junior High, and the new South High School.

The school campus has a significant impact on the corridor. Contributing factors include: 1) a large segment of young people walking to school and 2) young drivers going to and from South High School. The new South High School will add approximately three quarters of a mile of a ten foot wide sidewalk, some of which is designated as a Greenway Connector, along West Allison Road from Walterscheid Boulevard to Pinto Lane via Cribbon Avenue.

Retail Uses – Along South Greeley Highway

Retail use takes place on the east end of the corridor at the intersection of South Greeley Highway and West Allison Road. Two entrances are located on West Allison Road from the Safeway grocery store shopping center. This section of right of way has been widened from 50 feet to 65 feet for approximately 400 feet to accommodate road improvements including the addition of curb, gutter and sidewalk on the north side. The pavement in this section of West Allison Road has been upgraded and is in relatively good condition. However, it is still limited to two twelve foot travel lanes. Widening this section of the roadway may entail moving the power lines on the south side of the highway. There are existing storage units on the south side of West Allison Road at this location. A retaining wall will be necessary along north property line of the storage units due to the large grade difference between the street and the units. As you progress west along West Allison Road from South Greeley Highway, the grade drops down to a drainage basin crossing or low point running south across West Allison Road. The grade change also presents a problem with site distance for ingress and egress from the adjacent residences and the retail area driveways.

Residential Area – Between South Greeley Highway and Walterscheid Boulevard

The residential area depicted in the following pictures shows the relationship of the homes to the location of the existing road. Right of way width is very narrow in this section, approximately 50 feet. The road surface is approximately 22 feet wide with a rough edge of pavement and a gravel shoulder that is not maintained. Direct driveways access onto West Allison Road and misaligned driveways are potential causes for vehicle accidents. Lack of site distance and speeding vehicles compound the issues with the driveways. Pedestrian facilities along the road are nonexistent. Pedestrian and bicycle access is only on the roadway itself. There is no curb and gutter or sidewalks in this section of roadway.

Figure 8, Photos of Existing Residences





Residential Area – Between Walterscheid Boulevard and Snyder Avenue

There is uncontrolled access onto West Allison Road between Walterscheid Boulevard and Snyder Avenue at this time. The existing roadway includes one travel lane in each direction.

School Campus Area – Snyder Avenue west to Cribbon Avenue

This section of roadway consists of four twelve foot lanes plus parking lanes in an eighty foot right of way. Parking is allowed on both sides of the street. This section is directly between the new South High School and the existing Johnson Junior High. A new roundabout has been installed at the intersection of Snyder Avenue and West Allison Road.

Residential Area – Cribbon Avenue west to Arp Avenue

This section of the corridor has a medium density residential zoning. An apartment complex and single family residential homes line the street on both sides. The neighborhood is fully built out and is not conducive to widening of the road or right-of-way. Parking is primarily on-street. Many garages have been converted into living space. The approximate average distance from the front door to the edge of pavement is 36 feet, but would be reduced to less than 20 feet if the right-of-way was increased to 80 feet.. This section of roadway is currently classified as a local street with sixty (60) feet of right-of-way. The City standard for a collector street is eighty (80) feet of right-of-way. The speed limit is currently posted at twenty-five (25) miles per hour.

West Allison Road does not connect to Parsley Road on the west end. Two developed residential lots block the connection. If this connection were to be made, it would significantly increase traffic volumes through the residential portion of West Allison Road, a street heavily congested by on-street parking on each side. The street width meets Current City standards.

IDENTIFICATION AND EVALUATION OF ISSUES

ROAD ALIGNMENTS

Four horizontal alignment options were evaluated for the connection of the West Allison Road corridor to Parsley Boulevard

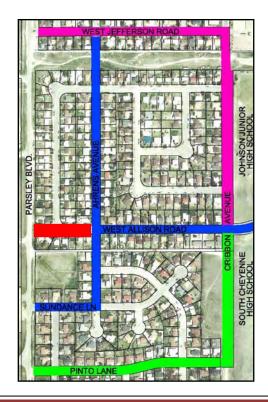
These options consisted of:

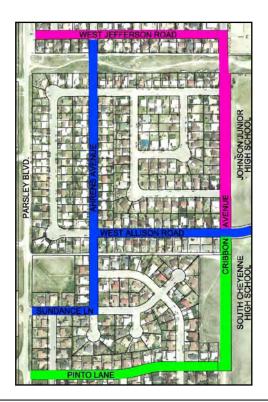
- 1. Pushing West Allison Road through the residential area for a direct connection to Parsley Boulevard
- 2. Using the existing West Jefferson Road via Cribbon Avenue
- 3. Using the existing Pinto Lane via Cribbon Avenue
- 4. Using the existing Ahrens Avenue north to West Jefferson Road and south to Sundance Lane via West Allison Road

The concerns with a direct connection through the residential area are the large impact to existing residences (which may require the purchase of multiple residential lots), and concern over the significant amount of traffic that would use the local residential road to travel east from Parsley Boulevard towards Walterscheid Boulevard if the street were opened up (connected to Parsley Boulevard as shown on Figure 9A). For these reasons, this direct connection was not recommended. It is recommended that the existing routes shown on Figure 9B be maintained until one or more of the following changes take place within the project area: the connection of West Allison Road to Southwest Boulevard is made, Parsley Boulevard is upgraded to an urban arterial section, and the area west of Parsley Boulevard develops.

Figure 9A, Road Alignment from Cribbon Avenue to Parsley Boulevard with West Allison Road extension to Parsley Boulevard

Figure 9B, Road Alignment from Cribbon Avenue to Parsley Boulevard





In addition to the connection at Parsley Boulevard, the alignment of a north-south connector road was also evaluated. This connector road would be located from Prosser Road on the south across West Allison Road near the midpoint between Walterscheid Boulevard and South Greeley Highway to Jefferson Road on the north. The exact location will be determined as development occurs. This connection was evaluated in accordance with the future road network as described in the Cheyenne Transportation Master Plan. The termination points, as well as the alignment will be finalized as development occurs. Figure 10 represents a possible location and alignment for the road. This road is approximately 2,600 feet long and would be classified as a local road with 36 feet of pavement and sidewalks in a 60 foot right of way.

Figure 10, North/South Connector (conceptual alignment)



A vertical alignment modification was also evaluated along West Allison Road to improve the stopping sight distance at crest vertical curves, especially where residential driveways exist. The crest condition just west of South Greeley Highway currently creates an unsafe condition for a number of residential drives. For this reason it is recommended that the grade at this crest condition be lowered and the slope on either side of the crest be reduced.

See appendix B, Sheets C-275 – C-278 for the plan and profiles for the corridor.

RIGHT OF WAY

The right-of-way for the West Allison Road corridor varies from 50 feet wide between South Greeley Highway and Walterscheid Boulevard, to 80 feet wide between Walterscheid Boulevard and Cribbon Avenue, to 60 - 70 feet wide within the residential areas on the west end of the corridor. Since West Allison Road is classified as a collector road, which requires an 80 foot right-of-way, an option to obtain additional right-of-way for the portions less than 80 feet was evaluated.

ROAD SECTIONS

A number of typical road sections were evaluated in choosing the best sections for the West Allison Road corridor. These sections ranged from rural sections with minimal pavement and drainage swales to full width City road sections for a collector including bike lanes and detached sidewalks. The road section recommended shows 36 feet of pavement with a six foot wide detached sidewalk. An additional 15 feet of right-of-way will be dedicated as development occurs along the south side of the corridor. A ten foot wide greenway connector will be constructed with the southern 15 feet of right-of-way west of the north/south connector street and an eight foot wide multi-use path east of the connector. See Appendix B for road sections used.

80' RIGHT-OF-WAY 15'-0" OPTIONAL PHASED RIGHT-OF-WAY 15'-0" TREE LAWN TREE LAWN 6'-0" 6'-0" 12'-0" 6'-0" 10'-0" 4'-0" TRAVEL LANE SIDEWALK SHOULDER TRAVEL LANE SHOULDER NORTH SOUTH

Figure 11. Proposed Typical Road Section

INTERSECTIONS

West Allison Road & South Greeley Highway

This is a signalized intersection of a County road and a State Highway. This intersection currently contains a high volume of traffic which will continue to grow as South High School opens and new developments build along the corridor and within the area. In order to anticipate this and maintain an acceptable level of service, improvements are recommended at this intersection. These include adding a dedicated left turn lane (northbound) with a sensor controlled signal, retiming the signals, and providing bicycle lanes and sidewalks.

West Allison Road & Walterscheid Boulevard

This is a signalized intersection (part of South High improvements). The corridor east of this intersection is controlled by Laramie County, while the area to the west is under City of Cheyenne control. With the South High School construction, the west side of the intersection will be improved to include turn lanes, drainage and signalization. According to the Baseline Corridor Study, prepared by Western Research and Devleopment, this intersection will see a decrease in the level of service in the next four years as a result of increased traffic volumes. A traffic signal is currently being constructed to accommodate the anticipated traffic increase from South High School and new developments in the area. The north and west legs of the intersection have also been improved. The full 80-feet of right-of-way will be needed to complete the intersection improvements. (Further improvements to the west leg of the intersection and possible relocation of the signal poles may be necessary for the proposed improvements to the east leg.)

West Allison Road & Desmet Avenue

This intersection is stop sign controlled on Desmet Avenue. West Allison Road at Desmet Avenue has been improved with the South High School improvements. No additional improvements to this intersection are recommended as part of this study.

West Allison Road & Snyder Avenue

This intersection is a roundabout (part of South High improvements). The roundabout was built as part of the South High School improvements, and will provide a higher level of service than a signalized intersection. No additional improvements are recommended at this intersection as part of this study.

West Allison Road & Johnson Jr. High School exit

This T-intersection is stop controlled on the north leg. Due to the amount of vehicles turning from westbound West Allison Road onto Cribbon Avenue and the short distance between the two intersections, there is some concern about the left turn exit onto West Allison Road. A second exit onto Cribbon Avenue would reduce the traffic turning onto and crossing West Allison Road at this point. If the number of vehicles exiting Johnson Jr. High stack up too long, it may be necessary to restrict this exit to right out only and use the second exit onto Cribbon Avenue and/or provide a secondary exit onto Cribbon Avenue for all travel eastbound on West Allison Road. See Sheet C-279 in Appendix B that shows the proposed improvements.

West Allison Road & Cribbon Avenue

This is a four way stop sign controlled intersection (the south leg part of South High School improvements). The collector road corridor turns south here and connects to Parsley Boulevard via Pinto Lane. This intersection will see additional vehicular trips with the connection to Pinto Lane and with the addition of South High School. As a result, the level of service will decrease if the four way stop control is kept in place. A roundabout would be recommended at this location to improve the level of service and safety of this intersection. In addition, the use of intersection narrowing or curb extensions would provide traffic calming and a shorter pedestrian path across the travel lanes of the intersection.

Cribbon Avenue & Pinto Lane

There will be continuous flow traffic though this 90 degree bend. The connection between West Allison Road and Pinto Lane was made as part of the South High School improvements. This intersection did not exist prior to the South High School improvements, and will generate additional traffic down Pinto Lane through the residential neighborhood and connecting to Parsley Boulevard. With the additional traffic volumes on Pinto Lane, it is recommended that the three cross streets with Pinto Lane, Trail Way Road, Red Rock Road, and Elkhorn Drive have stop signs placed on the north and south legs.

Pinto Lane & Parsley Boulevard

This T-intersection is stop sign controlled on Pinto Lane only. The number of vehicle trips at this intersection will increase as a result of the addition of South High School as with the Cribbon Avenue connection being made, however, since it is one of three connections to Parsley Boulevard, additional improvements are not justified at this time. This should be evaluated when Parsley Boulevard is improved.

Cribbon Avenue & Jefferson Road

This T-intersection is stop sign controlled on Jefferson Road eastbound and Cribbon Avenue southbound. With the proposed improvements to Goins Elementary School, which will provide an off street drop off lane for the school buses, this intersection's level of service should be improved without additional improvements to the intersection itself.

Jefferson Road & Parsley Boulevard

This T-intersection is stop sign controlled on Jefferson Road only. The number of vehicle trips at this intersection will increase as a result of the addition of South High School, however, since it is only one of three connections to Parsley Boulevard, additional improvements are not recommended at this time. A southbound left turn lane from Parsley Boulevard southbound to Jefferson Road eastbound is being constructed with the South High improvements. The type of intersection should be evaluated when further improvements are made to Parsley Boulevard.

TRAFFIC

A Synchro Traffic Analysis was performed for the existing signal at the West Allison Road/South Greeley Highway intersection and the proposed signal at the West Allison Road/Walterscheid Boulevard intersection to determine if a delay would be caused by the close proximity of the two intersections. Analyses were prepared for the Years 2014, 2019, and 2024. The Level of Service for these intersections remained acceptable for all conditions. Levels A-C have been designated as acceptable in the Highway Capacity Manual.

Table 2
Synchro Traffic Analysis

Intersection	Peak Time	Year	Average Delay	Intersection
			(s)	LOS
Allison/ Walterscheid	AM	2014	5.5	Α
Allison/S. Greeley Hwy	AM	2014	5.7	Α
Allison/ Walterscheid	PM	2014	5.4	Α
Allison/S. Greeley Hwy	PM	2014	7.7	В
Allison/ Walterscheid	AM	2019	5.6	Α
Allison/S. Greeley Hwy	AM	2019	5.8	Α
Allison/ Walterscheid	PM	2019	5.6	Α
Allison/S. Greeley Hwy	PM	2019	7.9	В
Allison/ Walterscheid	AM	2024	5.7	Α
Allison/S. Greeley Hwy	AM	2024	6.0	Α
Allison/ Walterscheid	PM	2024	5.7	В
Allison/S. Greeley Hwy	PM	2024	8.4	В

Speed tests were done for West Allison Road west and east of Walterscheid Boulevard, Cribbon Avenue north of West Allison Road, and Pinto Lane east of Parsley Boulevard. Since this was a concern of the local residents the test were done to have a base speed to compare to future speed tests. These tests were done in the spring of 2009 for the three western segments and the beginning of 2010 for the eastern segment. The average speeds are shown below in Table 3. The speed tests are included in Appendix D.

Table 3 Average Speeds

Major Roadway	Direction	Minor Roadway	Average Speed	% of vehicles exceeding
			(mph)	30 mph speed limit
Allison	west of	Walterscheid	33	99.7
Allison	east of	Walterscheid	33	96
Cribbon	north of	Allison	32	96
Pinto	east of	Parsley	26	12

TRAFFIC CALMING

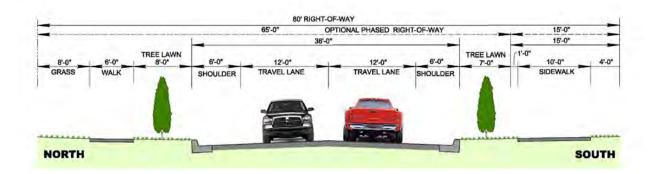
With the feedback from City and County officials and from the public during the three public meetings, a number of traffic calming devices were discussed for use on the West Allison Road corridor.

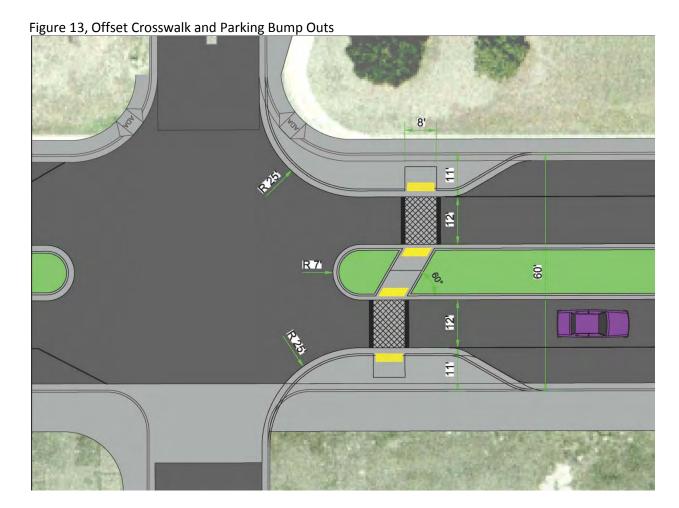
They are as follows:

- roundabout intersections
- center island narrowing with landscaping (choker islands)
- lane width narrowing (striping)
- intersection narrowing (curb extensions or bump outs)

These measures have been evaluated and implemented into the recommended road sections and roadway improvements as applicable. Landscape islands and parking lanes reduce the roadway width. An offset crosswalk with curb extensions is proposed east of the main entrance to Johnson Junior High School. This will provide pedestrians a safe haven for crossing the street along with requiring drivers only one twelve foot lane instead of 30 feet to cross. The landscape island and parking lane are shown below in Figure 12. The offset crosswalk and parking curb extensions are shown on Figure 8.

Figure 12, Median Island and Parking Lane Section





Although the use of landscaping in both the raised center islands and within the tree lawns between back of curb and sidewalk would enhance traffic calming, it can also provide aesthetic value for the corridor. The use of landscaping also has other benefits with regard to reduction of green house gases and urban heat island effect. For these reasons, the use of landscaping is highly recommended along this corridor.

DRAINAGE

The portion of the West Allison Road Corridor that this study covers for drainage is from South Greeley Highway to the east and Walterscheid Boulevard to the west. The remaining section of the West Allison Road Corridor has been improved as part of the development in the area. The corridor is located within the Allison Draw and Crow Creek Draw Drainage Basins. The division between the draws is Snyder Avenue.

A sub-basin between the new South High School and the existing Harmony Meadows subdivision drains north to West Allison Road and combines with a sub-basin north of West Allison Road and west of Walterscheid Boulevard. This flow combines at the intersection of West Allison Road and Walterscheid Boulevard and travels south on the west side of Walterscheid Boulevard.

When this corridor study was started, the area west of Walterscheid Boulevard and north of the sub-basin mentioned above, drained across Walterscheid Boulevard, through the proposed Gateway South project and south to West Allison Road. This area has been graded for a power sub-station with no

retention. The City of Cheyenne is requiring that the sub-station install some retention and it is assumed that the drainage flows from the sub-station will be brought back to the existing flow condition. This design has not yet been installed. For the analysis of the drainage report for the West Allison Road Corridor, the flow determined in the Gateway South study was considered to be the developed condition.

A sub-basin northeast of the intersection of West Allison Road and Walterscheid Boulevard drains south across West Allison Road and eastward to an existing low point in the street. A sub-basin north of West Allison Road will drain south across West Allison Road at a low point in the street west of South Greeley Highway. A portion of this sub-basin has recently been graded to drain into a storm drain facility that crosses under West Allison Road constructed with the Gateway South development to the north. A small developed sub-basin located northwest of the intersection of West Allison Road and South Greeley Highway drains east in West Allison Road to South Greeley Highway.

The Gateway South development has constructed a 48" RCP pipe crossing West Allison Road and an open channel to the south which connects to the Allison Draw.

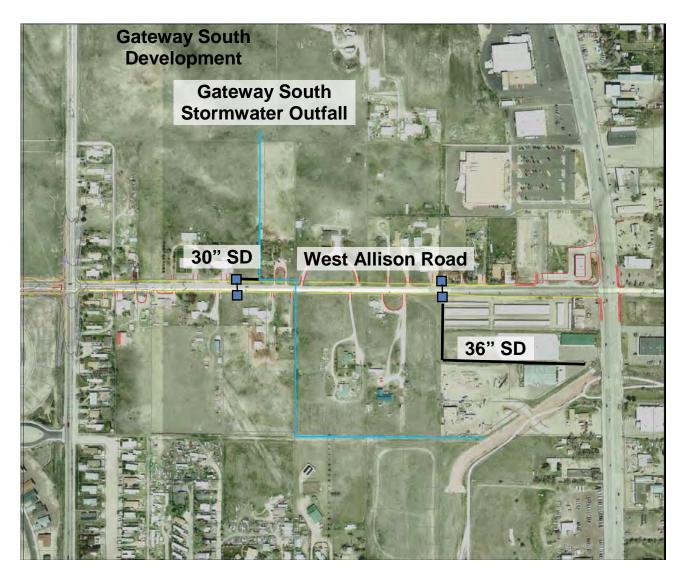
There are two options that have been analyzed for the discharge from the West Allison Road storm collection system. Option 1, as shown on Figure 14, will consist of roadway improvements with curb openings to allow flow to enter and exit the road in a manner consistent with the existing condition. This option does not improve the drainage problems in this area and is considered the last option.

Figure 14, Drainage Option 1



Option 2, as shown on Figure 15, will consist of roadway improvements with drop inlets at the two low points in West Allison Road. The flow picked up by the west drop inlets will be directed into a storm drain system that connects to the drainage facility constructed with the Gateway South project. The flow in the existing facility will increase by 3 cfs for the 100-year storm event. A second set of drop inlets will pick up flow at the low point in West Allison Road west of South Greeley Highway and direct the flow south in an existing South Cheyenne Water and Sewer District utility easement and east in an existing drainage easement to the Allison Draw. Permission to construct this facility within the utility easement is required from the District. Concerns regarding utility maintenance and infiltration into the sewer will need to be addressed for the District prior to their acceptance.

Figure 15, Drainage Option 2



Detention ponds should be required with future development at the areas west of Harmony Meadows and northwest of the intersection of West Allison Road and Walterscheid Boulevard. Also, the WAPA power sub-station should be required to retain flows in the pre-development 5-year storm. These proposed detention areas are shown on Figure 16.

Figure 16, Recommended Detention Areas



UTILITIES

Potable Water

The potable water along the eastern portion of the West Allison Road corridor is served by South Cheyenne Water and Sewer District. Their system extends as far west as Walterscheid Boulevard. Between South Greeley Highway and Walterscheid Boulevard there is a six (6) inch water line and three (3) fire hydrants. Along Walterscheid Boulevard there is a six (6) inch and a fourteen (14) inch water line. The western end of West Allison Road is serviced by the City of Cheyenne Board of Public Utilities and currently does not have any water lines along West Allison Road from just east of Desmet Avenue to Walterscheid Boulevard. Water lines should be installed and or upgraded throughout this corridor to accommodate future development including stub outs for extensions along the north-south road connector that will be between Walterscheid Boulevard and South Greeley Highway.

Sanitary Sewer

The sanitary sewer system along the eastern portion of the West Allison Road corridor is served by South Cheyenne Water and Sewer District. An eight (8) inch sanitary line exists along West Allison Road between South Greeley Highway and Walterscheid Boulevard and flows east towards South Greeley Highway. The western end of West Allison Road is serviced by the City of Cheyenne Board of Public Utilities and currently does not have any sewer lines along West Allison Road from Cribbon Avenue east.

Reuse

The City of Cheyenne Board of Public Utilities (BOPU) also has a reuse system and is expanding it across the City, however, does not service the southwest portion of Cheyenne at this time.

Natural Gas

The natural gas needs of the corridor are served by the Cheyenne Light, Fuel and Power Company. There is a 2 inch line running from Greeley Highway to Cribbon Avenue along West Allison Road. This line is of "medium" pressure designed to serve all parcels along this corridor.

Electric

The electric needs of the corridor are supplied by Western Area Power Administration (WAPA) and served by High West Energy and the Cheyenne Light, Fuel and Power Company. A preference would be for all of these lines to be underground including service to the adjacent lots. If undergrounding these lines is not feasible due to cost, the relocation of the poles may also be a viable option. A request to the electric company to move their poles to the back of right of way would need to be made. Either option will allow the construction of a new road section through this portion of the corridor.

Telephone

Phone services are currently provided by Qwest with both buried and aerial locations along the corridor. Fiber optic lines are also in place along West Allison Road from Walterscheid Boulevard to Cribbon Avenue. There are currently no plans to run fiber along the remainder of the corridor to South Greeley Highway.

Cable

Cable services are currently provided by Bresnan Communications and are also located above ground. The preference would be for these lines and services to also be undergrounded. If the electric lines are buried, then the communications lines would also be buried.

GREENWAY TRAIL / PEDESTRIAN / BICYCLE FACILITIES

Pedestrian facilities in the study corridor include sidewalks, striped crosswalks at signalized intersections, and shared use paths. West Allison Road contains sidewalks on both sides of the street along the section of road from Cribbon Avenue to Desmet Avenue a portion of which will be built as part of the South High School project. Also part of the South High School construction is a 10 foot wide walk on the south side of West Allison Road from Desmet Avenue to Walterscheid Boulevard. Sidewalk on the north side of West Allison Road between Desmet Avenue and Walterscheid Boulevard should be required when that adjacent property is developed. There are currently no sidewalks on either side of the road along West Allison Road east of Walterscheid Boulevard

According to the City of Cheyenne Greenway map, West Allison Road west of Walterscheid is a designated bike route. However, West Allison Road east of Walterscheid Boulevard is currently not on the Greenway map since that section is in the County. Due to the narrow pavement and lack of bike lanes, cyclists are discouraged from using this section. There are no existing and viable bike routes in this area.

As part of the intent to create a contiguous safe corridor for all modes of transportation a continuation of the Greater Cheyenne Greenway trail from Walterscheid Boulevard to the Gateway South drainage facility that crosses West Allison Road is recommended. In the future, this Greenway trail will continue from this point, south and east adjacent to the drainage facility to the Allison Draw Greenway Trail. From the Gateway South drainage facility under West Allison Road to South Greeley Highway an eight foot wide multi-use path is recommended.

Figure 17, Greenway Trail



Since South High school is not yet open, and a safe continuous sidewalk or bike lane does not yet exist throughout this corridor, a level of service analysis for pedestrians or bicycles was not performed. It is believed that once the high school is open and the road improvements are completed, including sidewalks, bike lanes and street lighting, the use of these facilities including the Cheyenne Greenway trail will increase significantly for the length of this corridor.

TRANSIT SERVICES

The current transit system does not service the West Allison Road corridor between South Greeley Highway and Parsley Boulevard, Johnson Jr. High, or the future South High School. The Cheyenne Transit Service is currently pursuing the possibility of adding an additional bus route (#7) to serve the SW portion of Cheyenne including South High, Triumph High and the West Allison Road corridor. Bus stops and turnouts will need to be incorporated into the final design drawings for this corridor.

WEST ALLISON ROAD - PUBLIC PARTICIPATION PROCESS

Three public meetings were held to inform and request comments from the neighborhood resident's. These meetings were held at critical points throughout the study and preliminary design process to allow ample public input that was used to evaluate the alterative road sections and alignments. Each meeting was advertised publicly and invitations were sent to attendees of prior meetings.

The first meeting was held on March 10, 2009 with the intent of informing the public of the project, projected timeline and overall process. During this meeting, valuable information was obtained from the local residents regarding the corridor. General corridor location maps and South High School information were provided. This meeting occurred during the existing conditions evaluation phase of the study and primarily asked the question, "What are your concerns with this corridor?"

The second meeting was held on August 5, 2009 with the intent of presenting roadway alternatives to the public and requesting comments on the desirability for each. A formal presentation was provided that included the different design options. An assortment of design elements, maps, street sections, streetscapes and additional materials were provided. The road sections, traffic calming options and alignment alternatives displayed were based on the input received at the first public meeting. This meeting occurred during the preliminary design phase of the study after the existing conditions were evaluated and the Baseline Report was complete.

The third meeting was held on January 26, 2010 with the intent of reviewing the final recommendations with the local residents, answering any questions they may have, and gaining their support for the project. The final recommendations included the roadway sections, alignment alternatives, pedestrian access, and drainage recommended for the corridor. This meeting occurred at the end of the preliminary design phase of the study and presented data and drawings created as part of the 35% design of the corridor.

A summary of the comments from each of these public meetings can be found in Appendix A. In addition to the public participation meetings, additional meetings were held with a Steering Committee consisting of the following:

Tom Mason - Cheyenne Metropolitan Planning Organization

Nancy Olson - Cheyenne Metropolitan Planning Organization

Doug Vetter - City of Cheyenne Engineering

Sam Berta – City of Cheyenne

Bob Nelson – City of Cheyenne

Tom Bonds - FHWA

Don Beard - Laramie County Public Works Director

Gary Kranse - Laramie County Planning Director

Matt Ashby - City of Cheyenne Urban Planning

James Sims - Cheyenne Metropolitan Planning Organization

Dennis Auker - Laramie County School District #1

The input of these mentioned steering committee members and other ensured that their concerns were addressed and that our recommendations ultimately reflected, not only the current local standards, but also the best solution to accommodate growth of the community.

CONSTRUCTION COSTS

A preliminary estimate of probable construction costs has been included in appendix E. The costs included right-of-way acquisition, roadway improvements, utilities, drainage, and landscaping. The preliminary probable costs totaled approximately \$2,974,000. The probable costs will need to be refined with further development of the civil improvement plans.

EXECUTIVE SUMMARY AND RECOMMENDATIONS

The corridor study not only involved a traffic study, drainage, report, and alignment study, but also the evaluation of the corridor surroundings containing residential use, some commercial use and the Laramie County School District # 1 Campus with Goins Elementary School, Johnson Jr. High School, and the new South High School which opens in 2010.

Recommendations

Listed below are the project recommendations:

- 1. West Allison Road will be constructed within a 65-foot right-of-way which will include a 36-foot wide roadway section, back of curb to back of curb. This right-of-way will be modified to 80-feet as development occurs. The additional 15-feet will be on the south side of the roadway.
- 2. An intersection will be constructed on West Allison Road, midway between South Greeley Highway and Walterscheid Boulevard. This will be an intersection with West Allison Road and a north/south connector street. The connector street will connect West Jefferson Road to the north and West Prosser Road to the south. The final location of this intersection should be determined at the time of final design and by development along the corridor.
- 3. A six foot sidewalk should be constructed on the north side of West Allison Road between South Greeley Highway and Walterscheid Boulevard.
- 4. A ten foot greenway should be constructed on the south side of West Allison Road between Walterscheid Boulevard and the north/south connector street. The greenway should eventually continue south and east and connect to the existing Greenway along the Allison Draw. This facility cannot be constructed until the south 15-feet of right-of-way has been dedicated.

- 5. An eight foot multi-use path should be constructed on the south side of West Allison Road between the north/south connector street and South Greeley Highway. This facility cannot be constructed until the south 15-feet of right-of-way has been dedicated.
- 6. The existing water and sewer utilities in West Allison Road between South Greeley Highway and Walterscheid Boulevard will need to be upgraded where necessary prior to roadway construction.
- 7. Regrade high point on West Allison Road approximately 700 feet west of South Greeley Highway to improve sight distance.
- 8. Install drop inlets in West Allison Road approximately 1000 feet east of Walterscheid Boulevard. Connect drop inlets into existing 42-inch RCP in West Allison Road to the east with a 30-inch RCP
- 9. Install drop inlets in West Allison Road at the low point approximately 1000 feet west of South Greeley Highway. These drop inlets will tie into the Allison Draw via a 36-inch RCP within an existing South Cheyenne Water and Sewer utility easement and a drainage easement.
- 10. It is recommended that detention ponds be installed with future development of these properties:
 - a. Northwest corner of the West Allison Road and Walterscheid Boulevard intersection.
 - b. Northeast corner of the property between South High School and Harmony Meadows.
 - c. Southeast corner of WAPA sub-station along Walterscheid Boulevard.
- 11. Add a right turn lane on the west leg of the West Allison Road and South Greeley Highway intersection. WYDOT will need to be coordinated with on the signal timing.
- 12. Construct landscaped median on West Allison Road between Snyder Avenue and Cribbon Avenue.
- 13. Construct bump-outs on West Allison Road on the east side of the main entrance to Johnson Junior High School. Construct offset sidewalk between the north and south bump-outs.
- 14. Stripe parking lanes on the north and south side of West Allison Road between Snyder Avenue and Cribbon Avenue.
- 15. Construct second exit from Johnson Junior High School onto Cribbon Avenue.
- 16. Install landscaped median on Cribbon Avenue north of Allison Road.
- 17. No changes will be made to West Allison Road between Cribbon Avenue and Parsley Boulevard. Modifications to this section should be evaluated at any of the following milestones: the connection of West Allison Road to Southwest Boulevard is made, Parsley Boulevard is upgraded to an urban arterial section, and the area west of Parsley Boulevard develops.
- 18. Install stop signs on the north and south legs of the intersections of Pinto Lane with Trail Way Road, Red Rock Road, and Elkhorn Drive.

A long term recommendation would be to evaluate the need for a roundabout at the intersection of West Allison Road and Cribbon Avenue. This evaluation should be done when a West Allison Road connection is made to Parsley Boulevard.

Execution of Recommendations

Since not all of the above recommendations are required at this time or have funds available for the improvements, it is recommended that these improvements be phased to address the primary concerns now and subsequent issues as they arise and funds become available. The timeframes for these phases of improvements should ultimately be determined by the City of Cheyenne, Laramie County, WDOT, and based on needs.

APPENDIX A

Public Participation Summary

West Allison Road Corridor Study

March 10, 2009 Public Open House Workshop # 1 Meeting Summary

On March 10, 2009 the West Allison Road Corridor Study held the first public workshop. The objectives of the meeting were to introduce the project and the process and to identify public issues and concerns. The attendees included residents and community organization representatives.

The attendees were asked to review several display boards showing the location of the study, future development, zoning, etc. and then respond to a questionnaire. The comments, questions and feedback received from the public will prove valuable in determining the issues and needs of the West Allison Road Corridor Study. The project team will review these issues and will present options and alternatives addressing them to the MPO and to the public in future public open houses.

QUESTIONNAIRE RESULTS

TOP THREE CONCERNS REGARDING WEST ALLISON ROAD

- Overall safety (pedestrian, bicycle, other)
- Speeding
- Traffic volume increase

Statistics:

- 1. Speeding is the single most common traffic rule violation and contributes to one third of all road traffic accidents. (W.H.O.)
- 2. Pedestrians account for about 30% of all traffic fatalities involving children under the age of 15 years. (NHTSA)
- 3. Road traffic crashes are the leading cause of death among children in the United States. (US Center for Disease Prevention)

TOP THREE TRAFFIC CONTROL DEVICES SELECTED

- 1. Radar Speed Sign
- 2. Roundabout Intersection
- 3. Speed Hump

Statistics:

- Most pedestrian fatalities occur in urban areas (73%) at non intersection locations (77%) in normal weather conditions (90%) and at night (67%). (NHTSA, 2007)
- Road traffic crashes are the leading cause of death among young people (ages 10-24) in the world. (WHO) They are also the leading cause of death among children in the United States. (United States Center for Disease Prevention)
- Speeding is the single most common traffic rule violation and contributes to one third of all road traffic crashes. (WHO)
- Traffic calming measures are a key intervention to road traffic crashes and deaths.
 (World Report on Road Traffic Injury Protection)
- If current trends continue, the number of people killed and injured on the world's roads

- will rise by more than 60% between 2000 and 2020. (WHO)
- In 2005, traffic accidents killed an average of 4 children under the age of 14 each day and injured 556 daily. In total, 203,000 children were injured and 1,451 were killed. (National Center for Health Statistics 2006 Report)
- 53% of fatal head injuries in an eight year study were to children who were playing in the street when injured (American Journal of Public Health)
- A study of 43 international traffic calming programs found that traffic calming solutions decreased traffic accidents by 8-100% (ITE Traffic Calming: State of the Practice)
- Two thirds of children who are hurt or killed in traffic accidents are struck and injured within several blocks (.25 miles) of their homes. (American Journal of Public health)
- Traffic calming has proven far more effective in preventing child pedestrian injuries than road safety education, which has been "unable to exert meaningful changes in the behavior of children". (American Journal of Public Health
- Residential areas within close proximity of speed control devices have 50% less pedestrian related vehicular collisions. (American Journal of Public Health)
- Speed humps were associated with a 53-60% reduction in injuries and fatalities among children struck by an automobile in their neighborhoods. (AJPH)
- Crash rates for roundabouts are 30-60% lower than signalized intersections, and are significantly less serious.
- Pedestrian and bicycle accidents are also significantly reduced for roundabouts compared to signalized intersections.

The public has issues with traffic volume and speed. They stated that speed on West Allison Road is a major issue, particularly near the schools. They felt that implemented traffic management solutions could improve the area and pedestrian safety while protecting neighborhood streets from excessive traffic.

Again, pedestrian and bicyclist safety was an issue. The public felt that the area along West Allison Road is not "walk-able" or "bike-able" because of the amount of vehicle traffic on West Allison Road and the lack of bicycle lanes in the street and lack of sidewalks.

West Allison Road – Notes

- 1. Easterly Portion South Greeley Highway to Walterscheid
 - a. 50' R/W (80' Required for Collector)
 - b. Numerous residences with multiple driveways onto West Allison Road
 - c. Overhead Power Lines on both sides of street
 - d. Drainage is a problem along West Allison Road (somewhat being addressed as part of the Gateway South Project) – Gene McDonald included additional flow for West Allison Road Improvements)
 - e. Looking at proposed location for north-south connector
 - f. Light at West Allison Road and Walterscheid Boulevard being done under South High School
- 2. Westerly Portion Cribbon Ave to Parsley Blvd
 - a. Narrow Street Section (50' R/W)
 - b. Number of Houses that back onto street (including apartment building)
 - c. Houses on end of block

Pinto cut through

West Allison Road Corridor Study – Public Feedback Summary

Public Meeting was held on March 10, 2009

23 Questionnaires were completed

The below information summarizes the resident's responses.

Existing Conditions

1. Do you believe Allison Road adequately accommodates:

a.	Traffic flow	Yes <u>8</u> (35%)	No <u>15</u> (65%)
b.	Pedestrian traffic	Yes <u>3</u> (13%)	No <u>20</u> (87%)
c.	Bicycles	Yes <u>3</u> (13%)	No <u>20</u> (87%)

Comments:

- Not wide enough, poor sidewalks for having (3) schools
- All of the above were not planned for resulting in today's inadequacies
- Pedestrians and cyclists need a dedicated path
- It does now, but I'm not so sure it will when South High opens
- At the present time it is ok, however, after the school opens I don't know.
- Traffic is too fast and not controlled
- 2. Do you feel speeding is an issue along the Allison corridor between South Greeley Highway and Walterscheid Boulevard? Yes 13 (57%) No 7 (30%) Not Sure 3 (13%) Comments:
 - People try to drive too fast for current conditions.
 - If the road is re-engineered, higher speeds would be safer (35 mph)
 - Yes, coming down the hill too fast
 - Yes, speed signs are too small
- 3. Do you feel speeding is an issue along the Allison corridor between Walterscheid Boulevard and Cribbon Avenue? Yes 10 (43%) No 8 (35%) Not Sure 5 (22%) Comments:
 - Not yet, but speeding will be an issue when the high school opens
 - Yes, a roundabout will help slow the speed
 - Speeding is an issue from Cribbon to Andrews
 - If emphasis is placed on bicycle and pedestrian safety, traffic speeds could be safely raised.
 - No, but needs road repair
 - Yes, it is a problem on Allison and elsewhere

In your opinion, will traffic speed reduction measures be necessary along Allison Road to help keep speeds down? Yes <u>18</u> (78%) No <u>1</u> (4%) Not Sure <u>4</u> (17%)

*If yes, then what type? Speed Hump <u>3</u> (10%) Speed Table <u>1</u> (3%)

Raised Crosswalk <u>2</u> (7%) Raised Intersection <u>0</u>

Textured Pavement 2 (7%) Roundabout Intersection 4 (15%)

Chicanes <u>1</u> (3%) Neckdown <u>0</u>
Center Island Narrowing 3 (10%) Choker 0

Radar Speed Sign 11 (38%) Speed Cushion 2 (7%)

Comments:

- Center Island Narrowing makes plowing difficult
- Concerned about Parsley/College intersection and bottle neck at Ames and Parsley/Deming now has a park there too. Need sidewalks all the way down Parsley.
- Cannot answer question without traffic data and planning goals.
- More pedestrians are moving between Cribbon and S. Greeley Hwy.
- Trash is being thrown from car windows
- Add radar speed sign with police patrols
- Road needs repair in front of Concord Village
- I don't like the idea of calming islands like on Vonderhor and 12th St.
- Needs greenway from Walterscheid to school (South and Johnson)
- Sight distance is too short coming from South Greeley Hwy towards Walterscheid
- Not sure the traffic calming will work with younger drivers, can we use enforcement?

South High School Addition

4. In what way do you think South High will impact Allison Road the most?

Speeding <u>13</u> (33%) Traffic Volume <u>22</u> (57%) Other <u>4</u> (10%)

Comments:

- Vehicle vs. pedestrian in emergency response
- Accidents
- Reckless drivers
- Student drivers
- I wonder about off campus smoking affecting my adjacent property

5. Are pedestrian facilities, bicycle facilities and crossings adequate on Allison Road?

Yes <u>2</u> (9%) No <u>21</u> (91%)

- Area was poorly planned and South High just added to the problem. I'm not against the high school, just the lack of planning.
- No, with the exception of the Snyder intersection and Walterscheid intersection
- No, particularly going down the hill
- They are ok now, but after school opens I don't know.
- No, Greenway upgrade will help

- No, there are no sidewalks
- No, bikes and pedestrians need sidewalks or paths

Future Growth

6. What issues do you see with the Pinto and West Jefferson connections from Allison Road to Parsley Boulevard?

Comments:

- It avoids the steep hill on Allison which is a good thing
- Will all streets be designed for present or future traffic?
- Traveling through residential areas
- Traffic and turning movements
- Traffic routed through residential street
- Pinto will have to deal with much higher vehicle traffic volumes, West Jefferson not so much
- Traffic volume
- Ok at present time
- Glad to see the Pinto connection
- More traffic
- More traffic than before schools open
- 7. Do you feel additional traffic from future growth in the area will impact your neighborhood? Yes 17 (74%) No 4 (17%) Don't Know 2 (9%)

- Yes, lower property values
- Yes, more traffic
- Yes, safety and noise
- Yes, traffic volume and speeding in a quiet neighborhood
- Yes, somewhat more cars
- Yes, there will be a distinct "honeymoon" period with South high school opening; our neighborhood is not used to the same problems as Education Boulevard and East Pershing Avenue
- Yes, just more cars on Allison and Cribbon
- Yes, there will be a lot more traffic
- Yes, more noise
- Yes, less peace and comfort
- Yes, more traffic and crime
- Yes, more people
- Yes, speeding

8. Does the intersection of West Jefferson and Cribbon (Goins Elementary) concern you for any of the following reasons:

a.	Pedestrian / Child safety	Yes <u>15</u> (65%) No <u>1</u> (4%)	Don't know <u>7</u> (31%)
b.	Traffic congestion	Yes <u>14</u> (61%) No <u>1</u> (4%)	Don't know <u>8</u> (35%)
c.	Visibility	Yes 12 (52%) No 2 (8%)	Don't know 9 (40%)

Comments:

- There is a park there also.
- Constant speeding in school zone
- The connection going East to West at Jefferson could be improved.

Additional Public Comments:

- Street lights need to be added to all surrounding residential streets due to increased traffic volume.
- I am concerned for tanker truck traffic on Parsley, need increased radius from Pinto; demand light during peak school hours or something needed.
- We live right behind Little Caesars and we have a rough time getting out of our driveway to get onto the road now. People don't appear to know what the stop sign says. They are barreling around the corner without a care whether we are half way out of our driveway or not. We have to pull back into our drive because they feel that they have the right to keep coming even with us half way into the street. They are always racing past our place well over the speed limit which is posted right in front of our place. There is a dip in the road to the West of us, which makes it so that we cannot always see the traffic coming up the hill. The speed is a problem.
- I don't understand how this concern is raised for input "after the high school is built" and why the planning dept. was late in doing so.
- I hope future meetings will provide info to me other than six boards that only show me the problems that I've lived with for 25 years.
- I want change because of poor past planning and maintenance of existing infrastructure.
- I hope this works out better than the roundabout. The snow removal is poor. The lane is not wide enough and busses are always climbing the curb to make the corner. It appears to be becoming a joke when the new school opens.
- The maximum speed limit should be no more than 30 mph between Sough Greeley Hwy and Walterscheid. There should be yellow flashing lights on Walterscheid to warn of a stop sign at the intersection of Allison Road and Walterscheid and to help slow traffic.
- Routing traffic through Pinto helps to decrease traffic in front of Goins School.
- How many feet of ROW do you plan to take?
- I hope this new high school does not affect me adversely, particularly from a littering standpoint.
- Allison Road needs to be replaced. The repair that has been done makes the street extremely rough.
- I would like to see some sort of natural walk between Johnson Jr. High and the New South High for safety reasons.
- Speed limit on Allison should be 30 mph.
- Some sort of school zone speed limit should be installed between Snyder and Cribbon (20 mph)

- If the roundabout fails (too many accidents when South High opens) a traffic light should be installed for safety reasons.
- Allison Road West, from Parsley to Southwest Drive? Bridge or tunnel over/under Burlington Northern Tracks for future growth.
- I would like to see more greenway linking Allison Road to Southwest Drive and linking to Central Cheyenne from Johnson / South High area.
- Respect and protect homes in Allison Road area and reroute Allison so as to not take homes from owners.
- Calming Islands do not work on the north, why not narrow the roadway and lower the speed limit instead.
- Right now, the speed limit signs are too small, too old, and drivers cannot read what is printed on them.

Review Notes:

- 1. If questions were left blank with no comments, their response was assumed to be "Don't know."
- 2. If a response was marked "no" but the comment indicated "yes," a Yes was tabulated.
- 3. If a response was marked "yes" but the comment indicated "no," a No was tabulated.
- 4. For question '4', the correct response was to check one of the three available options, however, many responses checked multiple boxes. For this reason, a percentage was calculated from the total number of responses rather than on the number of questionnaires submitted.

West Allison Road Corridor Study

August 5, 2009 Public Open House Workshop # 2 Meeting Summary

On August 5, 2009 the West Allison Road Corridor Study held the second public workshop. The objectives of the meeting were to provide proposed recommendations for the project and to receive public input. The attendees included residents and community organization representatives.

The attendees were given a presentation, showing the proposed road alignments and sections, drainage conditions, greenway trail continuation, Johnson Junior High Driveways, Project Schedule, and Traffic Calming. They were then asked to respond to a questionnaire. The comments, questions and feedback received from the public will prove valuable in determining residents concerns of the West Allison Road Corridor Study. The project team will review these issues and will present options and alternatives addressing them to the MPO and to the public in future public open houses.

QUESTIONNAIRE RESULTS

The public would rather have a bicycle path separated from traffic and would be more likely to use it if it was there. They would not be likely to use a new connection to Parsley Road but would like to see a continuous connection from West Allison Road to Parsley Boulevard. They are concerned with increased traffic. They would rather see a 65-foot right-of-way for West Allison Road from Walterscheid Boulevard to South Greeley Highway with street lights and underground power lines. Approximately half have drainage issues.

West Allison Road Corridor Study – Public Feedback Summary

Public Meeting was held on August 5, 2009

14 Questionnaires were completed

The below information summarizes the resident's responses.

If on-street bicycle lanes were provided along the West Allison Road corridor from Cribbon Ave to South Greeley Hwy, would you use them? If so, how frequently?
 Yes _3__(22%) No _9__(64%) None Given _2__(14%)

Comments:

- I'm not a biker
- Don't have a bicycle
- 3/week
- Weekly
- 2. Would you be more likely to use bike paths if they were separated from traffic? Yes _6__(43%) No _7__(50%) None Given _1__(7%)

Comments:

- Bike lanes are good. Shared walks may work.
- That mixes commuting bike traffic w/ walkers, joggers dog walkers etc.
- 3. If sidewalks and /or the Greenway Paths were installed along the West Allison Road corridor from Cribbon Ave to Greeley Hwy, would you use them? If so, how frequently?

 Yes _8 _ (57%) No_5 _ (36%) None Given _1 _ (7%)

- 2 times a week
- Often
- Possible walk
- Everyday
- Weekly
- Whenever I walked

4.	A road connection is being built from West Allison Road at Cribbon Ave south to Pinto Lane.
	This will provide a new connection to Parsley Boulevard Do you plan to use this connection? If
	so, how frequently?

Comments:

- 20 year
- 4-5/week
- Occasionally. I don't often go that way
- 5. In addition to the Pinto Lane and West Jefferson connection to Parsley Blvd, would you like to see a continuous connection from West Allison to Parsley Blvd? Of the three routes, which route would you most likely use?

Comments:

- if you put Allison through to Parsley you increase traffic
- I live on West Allison
- W. Allison to Parsley
- Definitely! Daily
- West Jefferson
- Connection from W. Allison to Parsley
- Allison
- Allison. Traffic will come no matter what. Most direct route makes most sense.
- 6. Two different street sections are being considered for the South Greeley Hwy to Walterscheid Blvd portion of the project. Would you prefer to see: (See Proposed Corridor Road Sections Display Board)

Option 1 (80' Right-of-Way with a detached multi-use path and detached sidewalk with landscaped tree lawns on both sides)

Option 2 (65' Right-of-Way with an attached multi-use path on the south and detached sidewalk with a landscaped tree lawn on the north side only)

Comments:

- [Option 1] NO WAY!
- [Option 2] I'm O.K. with this, but it is NOT shown that way on Option 2 map.
- 50' better
- 7. Would you like to see the entire West Allison Road corridor lit with street lights? Yes _11_(79%) No _2__(14%) None Given _1__(7%)

Comments:

- Certain sections, not entire stretch
- 8. Would you like to see the overhead power lines along West Allison Road moved underground? Yes _6__(43%) No _5__(36%) None Given _3__(21%)

Comments:

- Not concerned
- Doesn't make much difference
- Doesn't matter/Either way is fine.
- No opinion
- 9. Within the past year, have you experienced any rain water, snow melt, or flooding problems along the West Allison Road right-of-way? If so, please describe.

Yes _7__(50%) No _6__(43%) None Given _1__(7%)

- Corner of Ahrens & Allison
- The alley south of our property gets a lot of water in our yard
- N/A
- Ice-Slick
- Snow melt & plowing [our] drive way shut

Additional Public Comments:

- What are you doing about mail boxes? I would be ok with a community mailbox system if it isn't too far from my home.
- I want to keep my horses and livestock without restrictions. I would also like to see a bridle or horse trail on the south side of W. Allison Rd between S. Greeley Hwy and Cribbon.
- I want you to consider ditches (depressed medians) in the center of W. Allison Rd instead of fancy landscaping. This will help with flooding and snow removal, and will also slow down traffic.
- Consider taking Cribbon to College
- Planting strips are very difficult to maintain water, snow, etc.
- Why shoulders?
- Wider travel way makes for more speed Allison is a raceway now.
- Calming islands are poor solutions to slowing traffic.
- Sidewalk on one side only.
- We have trouble getting in and out of our driveway. My concern is if you lower the road in front of us, what is our access going to be.

West Allison Road Corridor Study

January 26, 2010 Public Open House Workshop # 3 Meeting Summary

On January 26, 2010 the West Allison Road Corridor Study held the third and last public workshop. The objectives of the meeting were to provide proposed recommendations for the project and to receive public input. The attendees included residents and community organization representatives.

The attendees were presented drawing boards, showing the overall project recommendations, recommended road alignments and sections, recommended improvements to West Allison Road between Snyder Avenue to Cribbon Avenue, and the recommended drainage improvements. The 35% plans were also available for review. MPO and Western Research and Development staff were available to answer questions regarding the boards and the project. The public was asked to fill out a comment card. The comments, questions and feedback received from the public will prove valuable in determining resident's opinions on the final project recommendations of the West Allison Road Corridor Study.

COMMENT CARD RESULTS

Mr. Ed Murray, III requested that the recommendation for the roundabout at the intersection of West Allison Road and Walterscheid Boulevard be removed. After conversation with City of Cheyenne staff this recommendation has been removed. A copy of the memorandum of the comments is included in this Appendix.

Other comments included concerns regarding the street lighting and snow accumulation at the existing roundabout on West Allison Road and Snyder Avenue and the construction of medians along Cribbon Avenue north of West Allison Road. There were also positive comments of satisfaction of the plan.

MURRAY PROPERTIES

Edward F. Murray, III - President 1616 Warren Ave., Ste. 21 Cheyenne, Wyoming 82001

TELEPHONE (307) 634-8364 CELL PHONE (307) 421-2741

E-MAIL: Murrayproperties@qwestoffice.net FAX NUMBER (307) 634-2413

MEMORANDUM January 28, 2010

TO:

Gary Grigsby

Western R & D, Ltd.

SENT VIA FAX (307)635-0410

FROM:

Ed Murray, III

Murray Properties

RE:

Allison Road Corridor Study

Dear Gary:

It was nice visiting with you a few minutes ago on the telephone. As we discussed, on January 26, 2010 I attended the open house at JJHS in regards to the Allison Road Corridor Master Plan and was surprised to see that one of your display boards indicated that a round-a-bout was being recommended for the intersection of Walterscheid and Allison Rd. This note shall confirm that when we spoke a few minutes ago, you assured me that no round-a-bout would be recommended for that intersection inasmuch as the matter has already been decided with the powers to be and it was determined to have that intersection (Walterscheid and Allison Rd.) remain a straight intersection with the control lights which are in the process of being installed. In any event you stated that you would make sure that any final reports regarding this study correctly describe the intent for that intersection.

On the other note regarding my receipt of a copy of the drainage plan, please go ahead and send it to me in digital format using my e-mail address as shown above on this letterhead.

Very Truly Yours,

Edward F. Murray

President

MURRAY PROPERTIES

EFM:via

1 .9 0888.oN

APPENDIX B

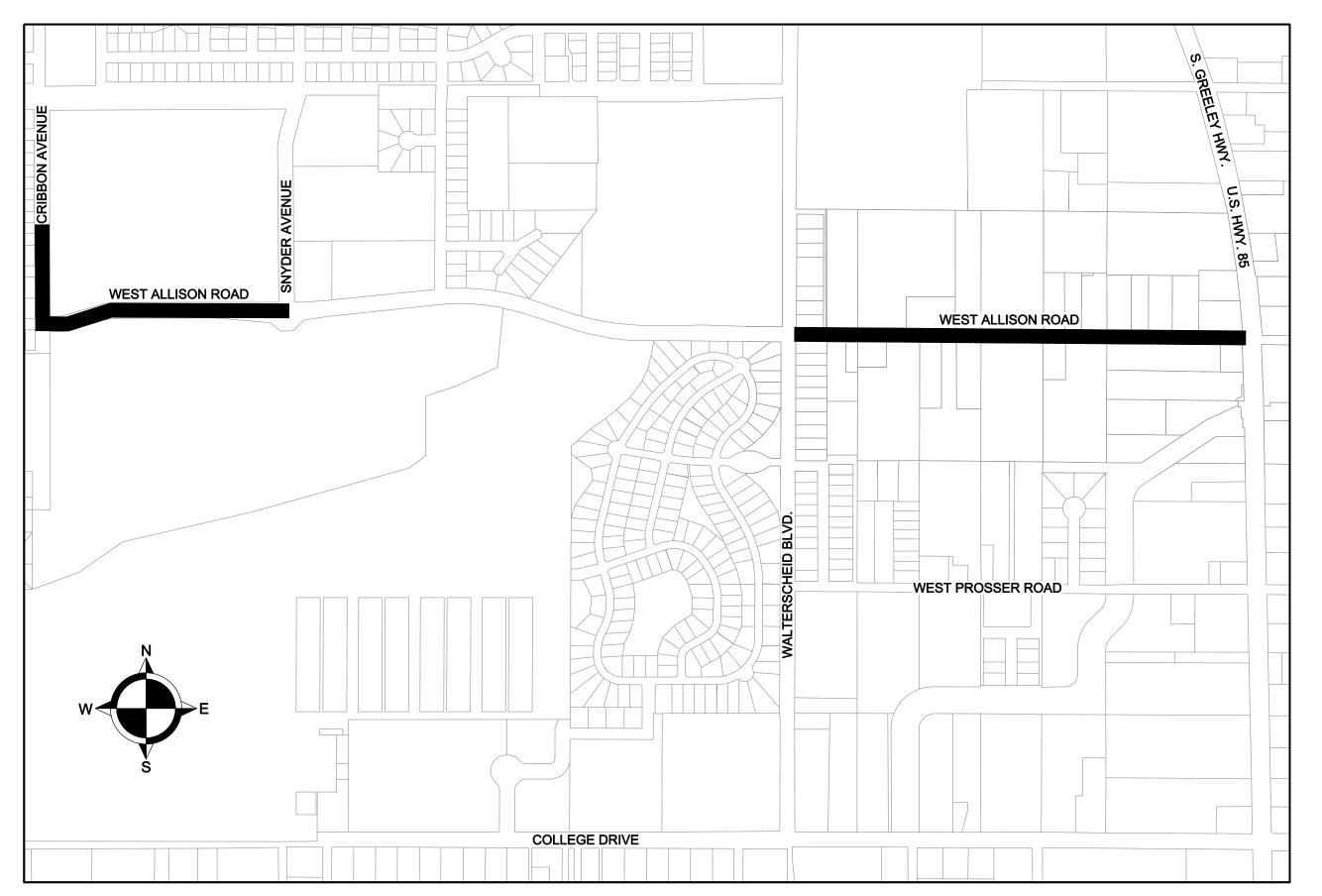
Preliminary Design

PRELIMINARY DESIGN AND CONSTRUCTION PLANS FOR

WEST ALLISON ROAD IMPROVEMENTS

CHEYENNE, WYOMING MARCH 17, 2010

GENERAL NOTES:



VICINITY MAP



PROJECT CONTACTS

PROJECT MANAGER **DAVID TRUSHAW** (307)632-5656 dtrushaw@wrd-ltd.com

LAND SURVEYOR **DAVID SWANSON** (307)632-5656 dswanson@wrd-ltd.com

SHEET INDEX

C-010	TYPICAL SECTIONS
C-100	EXISTING CONDITIONS PLAN
C-120	RIGHT-OF-WAY PLAN
C-275 - C-278	WEST ALLISON ROAD PLAN & PROFILES
C-279	JOHNSON JR HIGH PEDESTRIAN CROSSING
C-290	ALTERNATE INTERSECTION (ROUNDABOUT

COVER SHEET

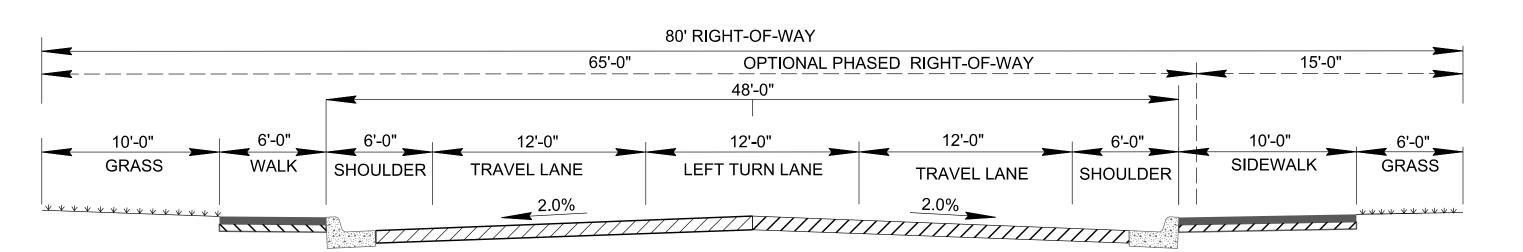
ENGINEER'S CERTIFICATE

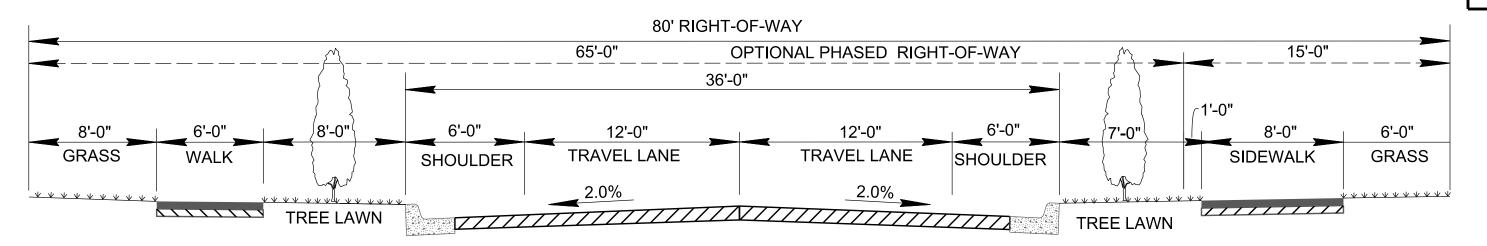
I, DAVID W. TRUSHAW, A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF WYOMING, DO HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED UNDER MY DIRECT SUPERVISION AND ARE CORRECT AND ACCURATE TO MY BEST KNOWLEDGE AND BELIEF.

DAVID W. TRUSHAW, PE 12520

C-001

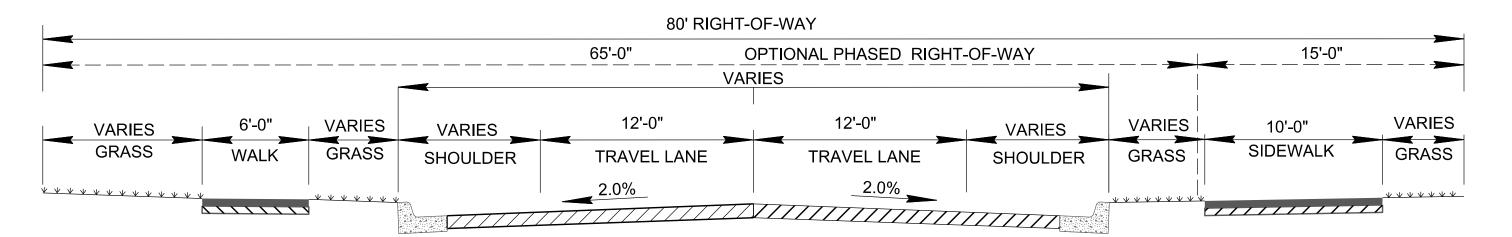
PROPOSED TYPICAL SECTIONS





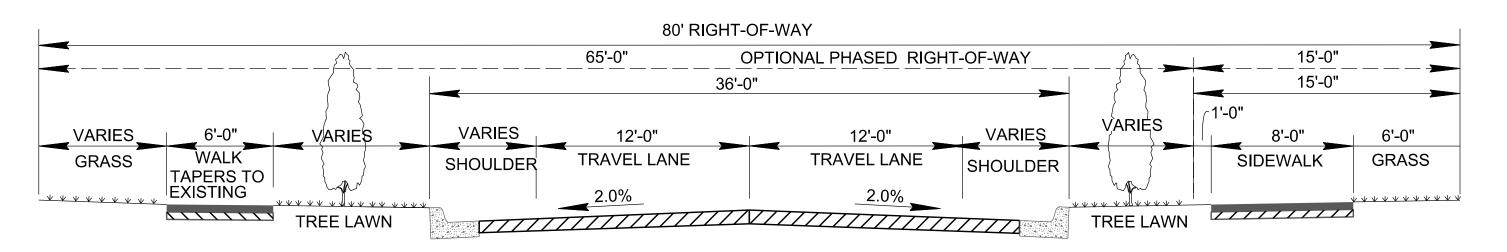
WEST ALLISION ROAD SECTION STA. 442+69.38 to STA. 445+06.99 (AT INTERSECTIONS)

SECTION 1



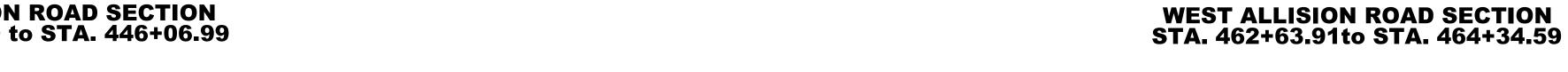
WEST ALLISION ROAD SECTION STA. 453+63.80 to STA. 462+63.91 (AT INTERSECTIONS)

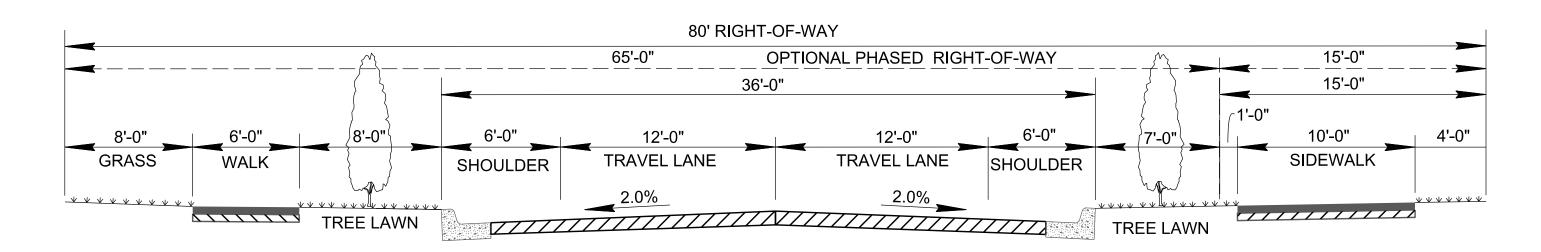
SECTION 4

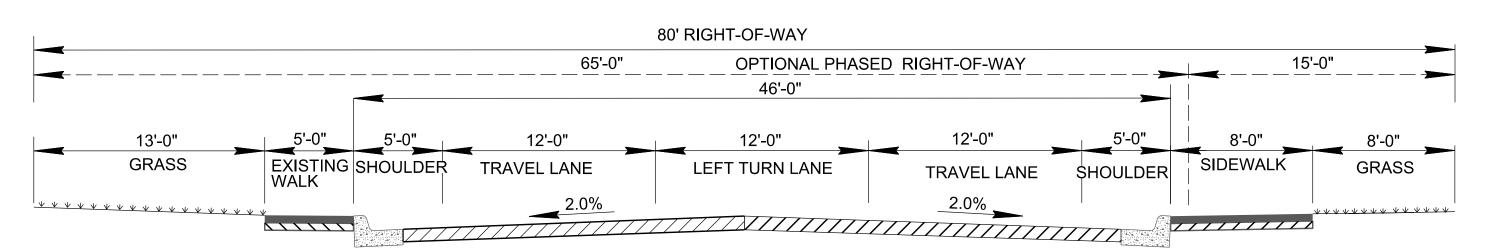


WEST ALLISION ROAD SECTION STA. 445+06.99 to STA. 446+06.99

SECTION 2







WEST ALLISION ROAD SECTION

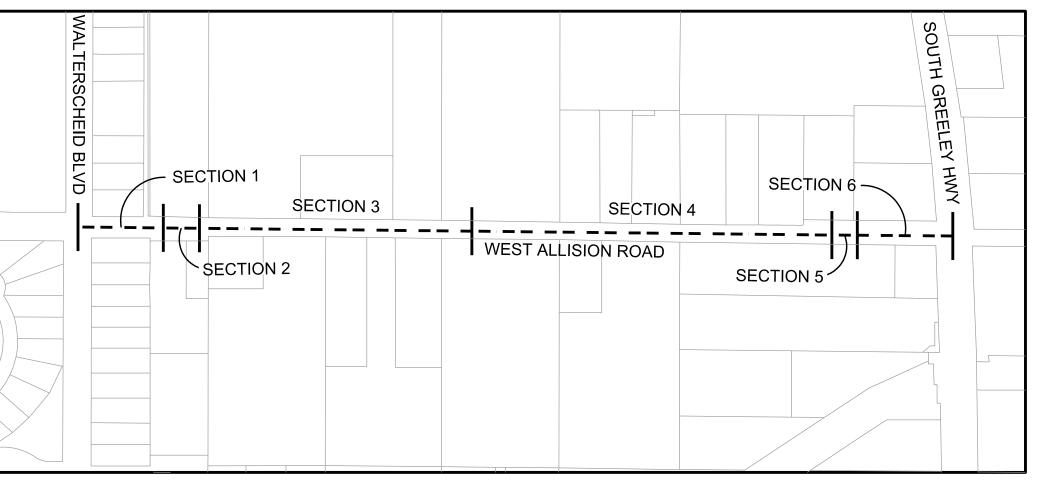
STA. 464+34.59 to STA. 467+00.00

SECTION 5

WEST ALLISION ROAD SECTION STA. 446+06.99 to STA. 453+63.80 (AT INTERSECTIONS)

SECTION 3

(AT INTERSECTIONS) **SECTION 6**



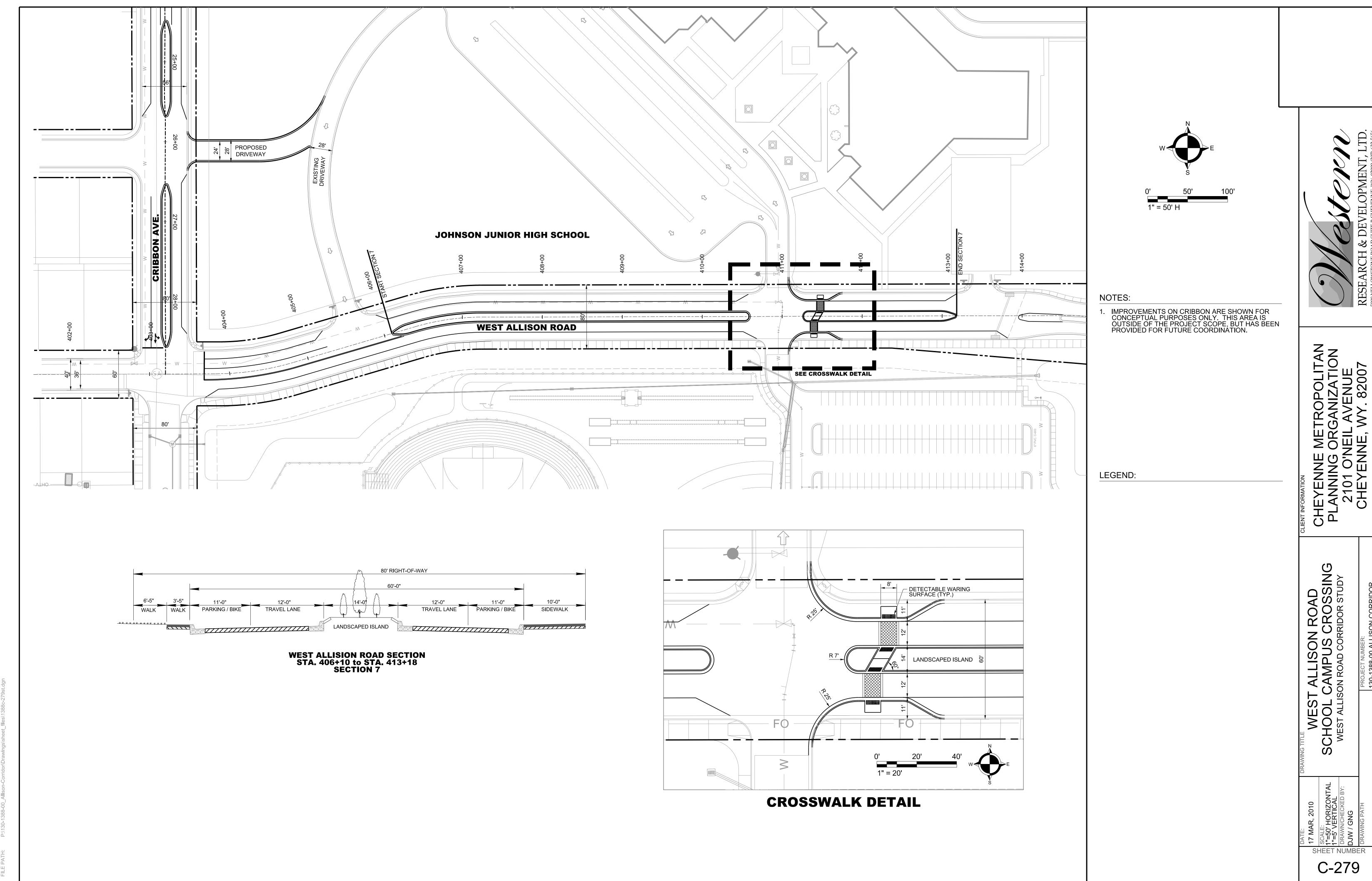
SECTION LOCATIONS

CTIONS

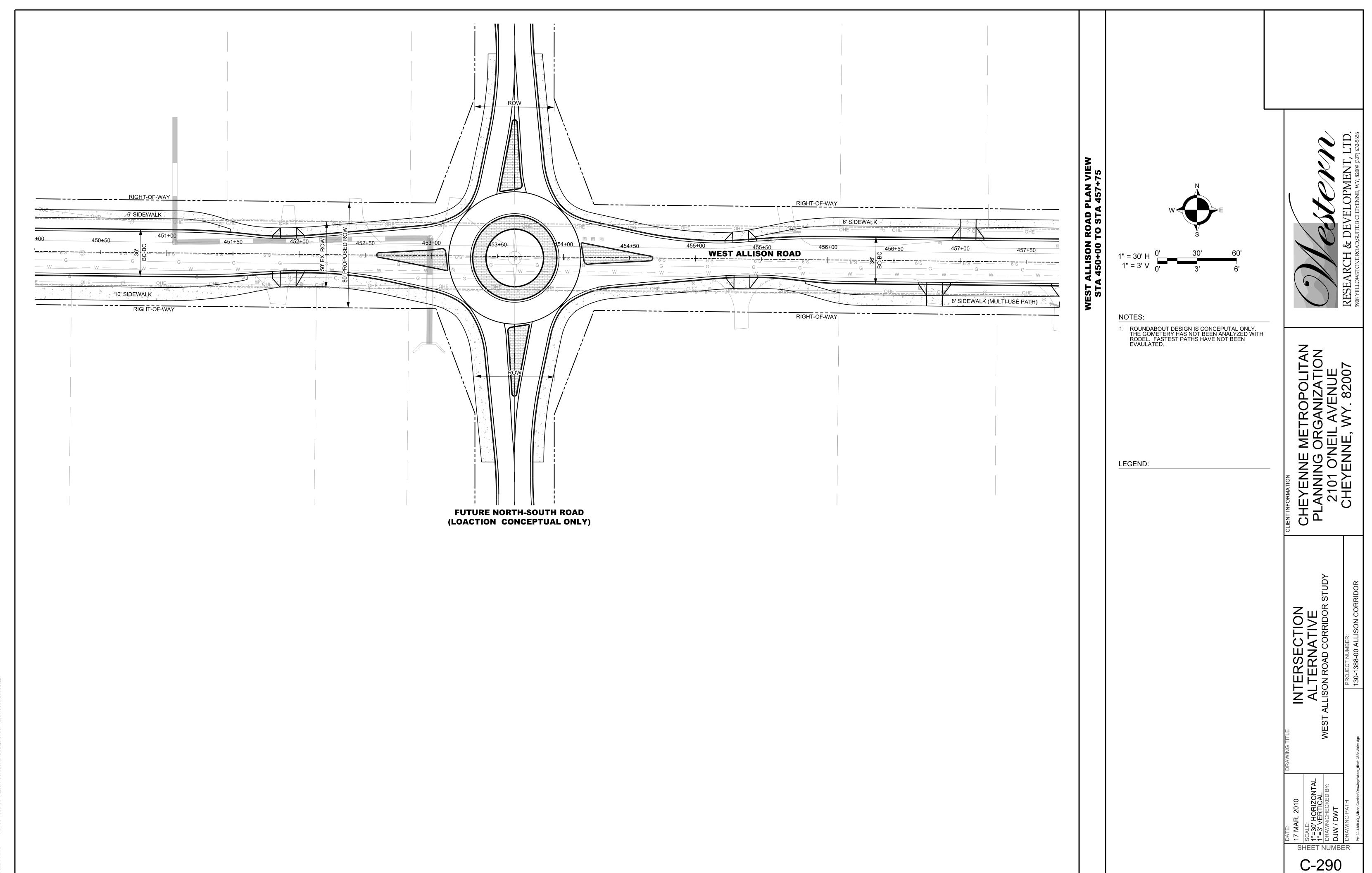
C-010

SHEET NUMBER



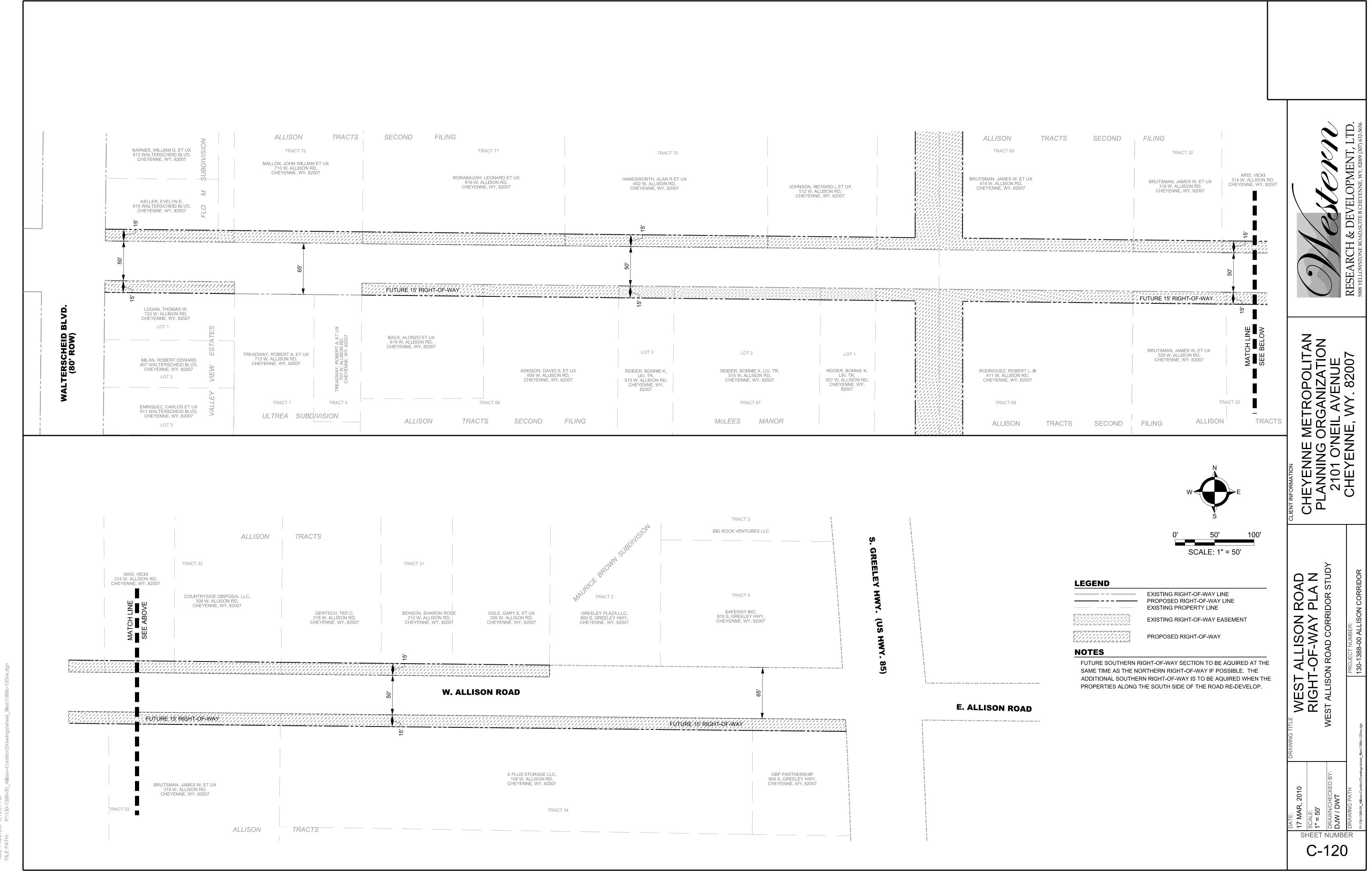


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APPENDIX C

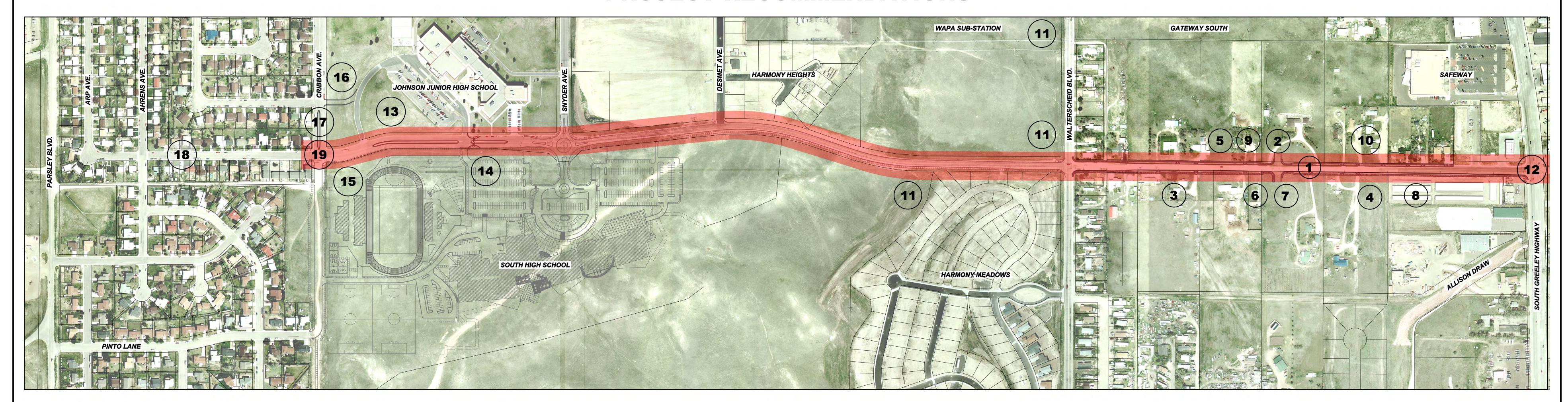
Survey Summary (ROW Map)



APPENDIX D

Final Meeting Boards

(SOUTH GREELEY HWY TO PARSLEY BLVD)
PROJECT RECOMMENDATIONS



PROJECT RECOMMENDATIONS (#)

- # RECOMMENDATION LOCATION
- ig(1ig) 65 FT RIGHT-OF-WAY MODIFIED TO 80 FT AS DEVELOPMENT OCCURS IN THE AREA
- 2 INTERSECTION FOR NORTH/SOUTH ROAD TO CONNECT WEST JEFFERSON ROAD AND WEST PROSSER ROAD
 - -INTERSECTION SHOULD BE LOCATED AT APPROXIMATE MID-POINT BETWEEN SOUTH GREELEY HWY AND WALTERSCHEID BLVD
 - -THE FINAL LOCATION SHOULD BE DETERMINED AT FINAL DESIGN AND BY DEVELOPMENT ALONG THE CORRIDOR
 -A ROUNDABOUT IS THE PEFERRED INTERSECTION CONTROL DEVICE
- (3) 10 FT GREENWAY CONNECTOR FROM WALTERSCHEID BLVD TO NEW INTERSECTION
 - -FROM NEW INTERSECTION, GREENWAY CONNECTOR SHOULD RUN SOUTH AND CONNECT TO EXISTING GREENWAY ALONG ALLISON DRAW
 - -CANNOT BE CONSTRUCTED UNTIL SOUTHERN RIGHT-OF-WAY HAS BEEN DEDICATED
- 8 FT MULTI-USE PATH FROM NEW INTERSECTION TO SOUTH GREELEY HIGHWAY
 -CANNOT BE CONSTRUCTED UNTIL SOUTHERN RIGHT-OF-WAY HAS BEEN DEDICATED
- (5) 6 FT SIDEWALK FROM WALTERSCHEID BLVD TO SOUTH GREELEY HWY
- 6 UPGRADE EXISTING WATER AND SEWER UTILITIES PRIOR TO REBUILDING ROAD BETWEEN SOUTH GREELEY HWY AND WALTERSCHEID BLVD
- (7) MOVE OVERHEAD UTILITES UNDERGROUND, IF POSSIBLE, OR FUTHER AWAY FROM THE ROAD
- 8 REGRADE HIGH POINT ON ALLISON ROAD APPROXMATELY 700 FT WEST OF SOUTH GREELEY HWY TO IMPROVE SIGHT DISTANCE
- 9 INSTALL DROP INLETS APPROXIMATELY 1000 FT EAST OF WALTERSCHEID BLVD AND CONNECT TO EXISTING STORM SEWER FACILITIES
- (10) INSTALL DROP INLETS AT LOW POINT BETWEEN SOUTH GREELEY HWY AND WALTERSCHEID BLVD (APPROXIMATELY 1000 FT WEST OF SOUTH GREELEY HWY)
 - CONVEY RUN-OFF FROM INLETS SOUTH TO ALLISON DRAW VIA STORM SEWER AND OPEN CHANNEL FACILITIES
- 11 DETENTION PONDS ARE RECOMMEND AT THE FOLLOWING LOCATIONS WITH THE FUTURE DEVELOPMENT OF THOSE PROPERTIES:
 - -NORTHWEST CORNER OF WEST ALLISON ROAD/WALTERSCHEID BLVD INTERSECTION
 - -NORTHEAST CORNER OF PROPERTY BETWEEN SOUTH HIGH SCHOOL AND HARMONY MEADOWS
 - -SOUTHEAST CORNER OF WAPA SUB-STATION ALONG ALONG WALTERSCHEID BLVD

- (12) ADD RIGHT-TURN LANE AT THE SOUTH GREELEY HWY INTERSECTION -COORDINATE WITH WYDOT TO TIME SIGNAL
- (13) INSTALL LANDSCAPED MEDIAN BETWEEN SNYDER AVE AND CRIBBON AVE
- 14) INSTALL BUMP-OUTS ON EAST SIDE OF MAIN ENTRANCE TO JOHNSON JUNIOR HIGH -CONSTRUCT OFF-SET CROSSWALK BETWEEN NORTH AND SOUTH BUMP-OUTS
- 15) STRIPE PARKING LANES ON NORTH AND SOUTH SIDES OF WEST ALLISON ROAD BETWEEN SNYDER AVE AND CRIBBON AVE
- (16) ADD SECOND EXIT FROM JOHNSON JUNIOR HIGH ONTO CRIBBON AVE
- (17) INSTALL LANDSCAPED MEDIAN IN CRIBBON AVE NORTH OF WEST ALLISON ROAD
- 18 MAKE NO CHANGES TO WEST ALLISON ROAD BETWEEN CRIBBON AVE AND PARSLEY BLVD -MODIFICATIONS TO THIS SECTION SHOULD BE EVALUATED AT ANY OF THE FOLLOWING MILESTONES:
 - +THE CONNECTION OF WEST ALLISON ROAD TO SOUTHWEST BLVD IS MADE
 - +PARSLEY BLVD IS UPGRADED TO AN URBAN ARTERIAL SECTION
 - **+THE AREA WEST OF PARSLEY BLVD DEVELOPS**

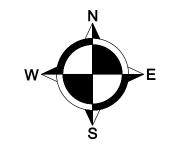
LONGTERM RECOMMENDATIONS

(19) EVALUATE ROUNDABOUT AT INTERSECTION OF WEST ALLISON ROAD AND CRIBBON AVE
-ANALYSIS SHOULD TAKE PLACE WHEN WEST ALLISON ROAD CONNECTION IS MADE TO PARSLEY BLVD



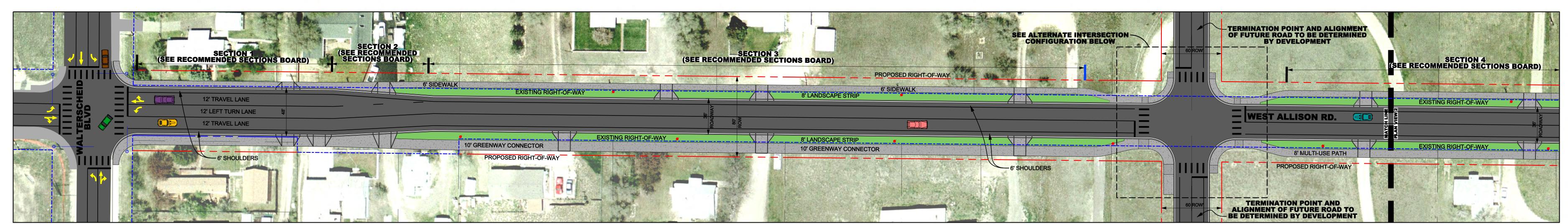


(WALTERSCHEID BLVD. TO SOUTH GREELEY HIGHWAY)
RECOMMENDED ALIGNMENT

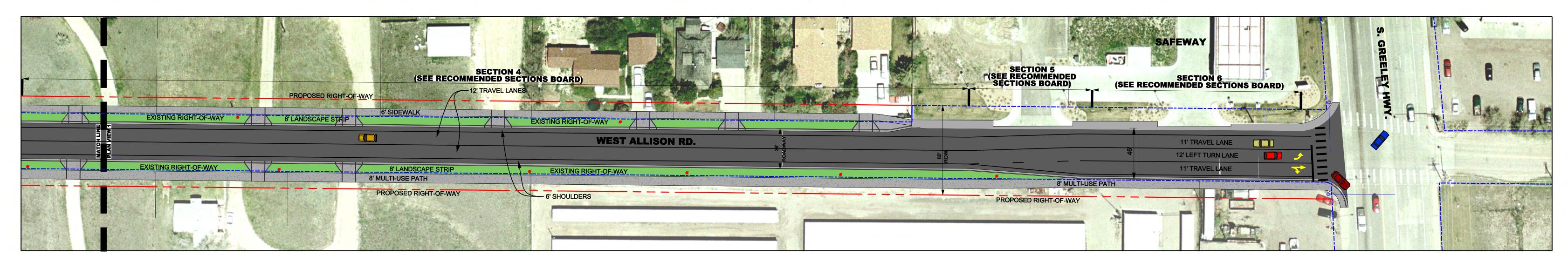




PROPOSED STREETSCAPE

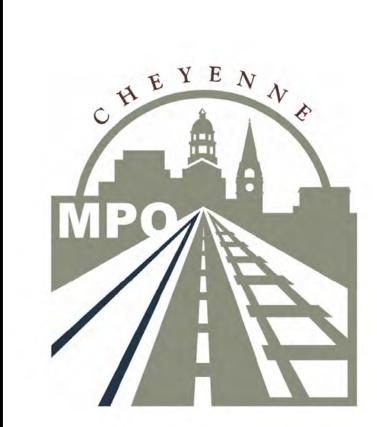


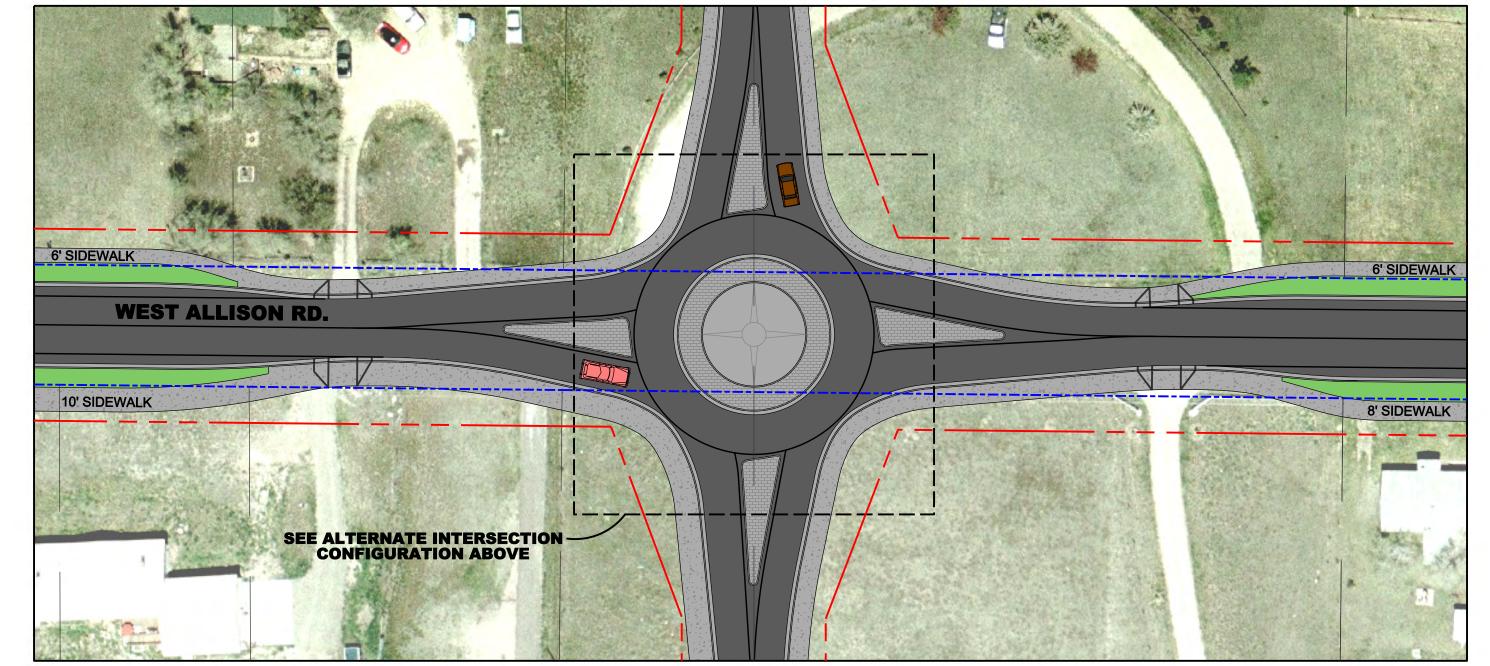
PLAN VIEW 1



PLAN VIEW 2

NOTE:
DRIVEWAY APPROACHES WILL BE EVALUATED ON AN INDIVIDUAL BASIS DURING THE FINAL DESIGN OF THE PROJECT

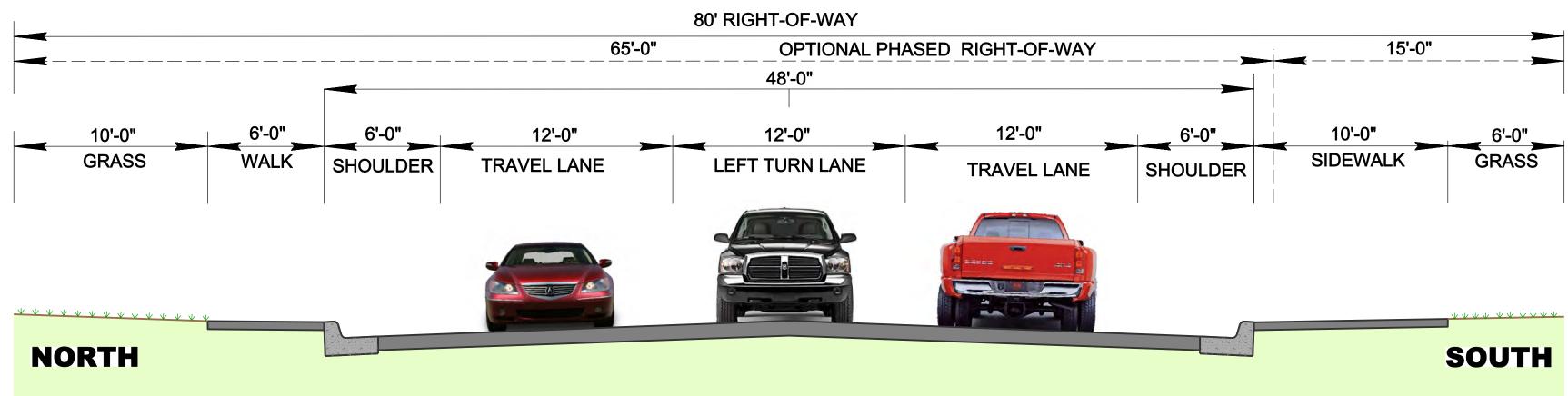




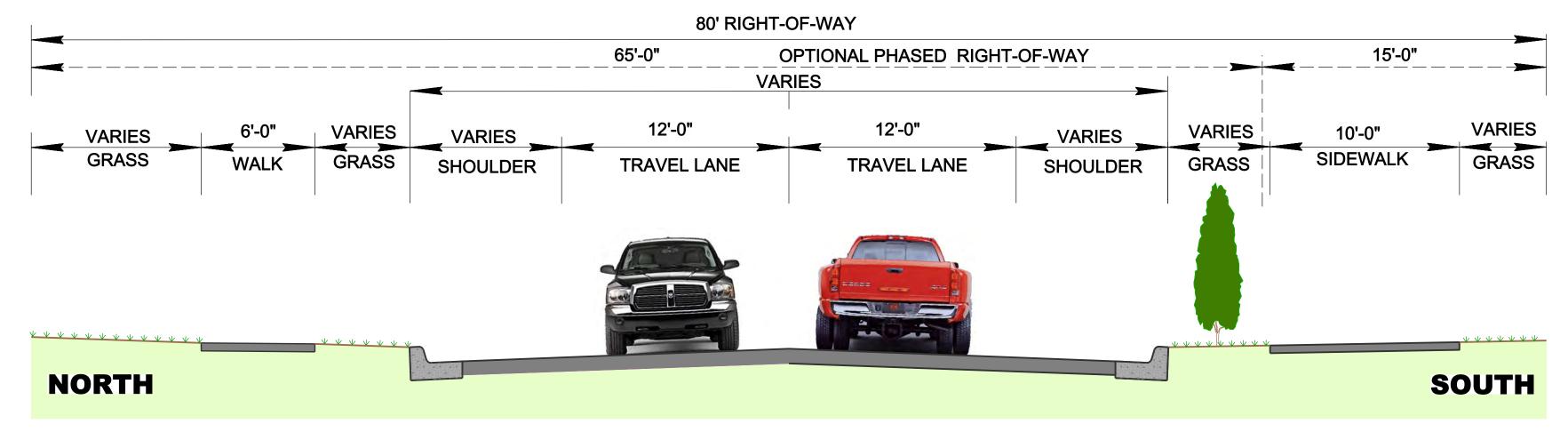
ALTERNATE INTERSECTION DESIGN FOR NORTH/SOUTH ROAD CONNECTION (EXACT LOCATION TO BE DETERMINED BY FUTURE DEVELOPMENT)



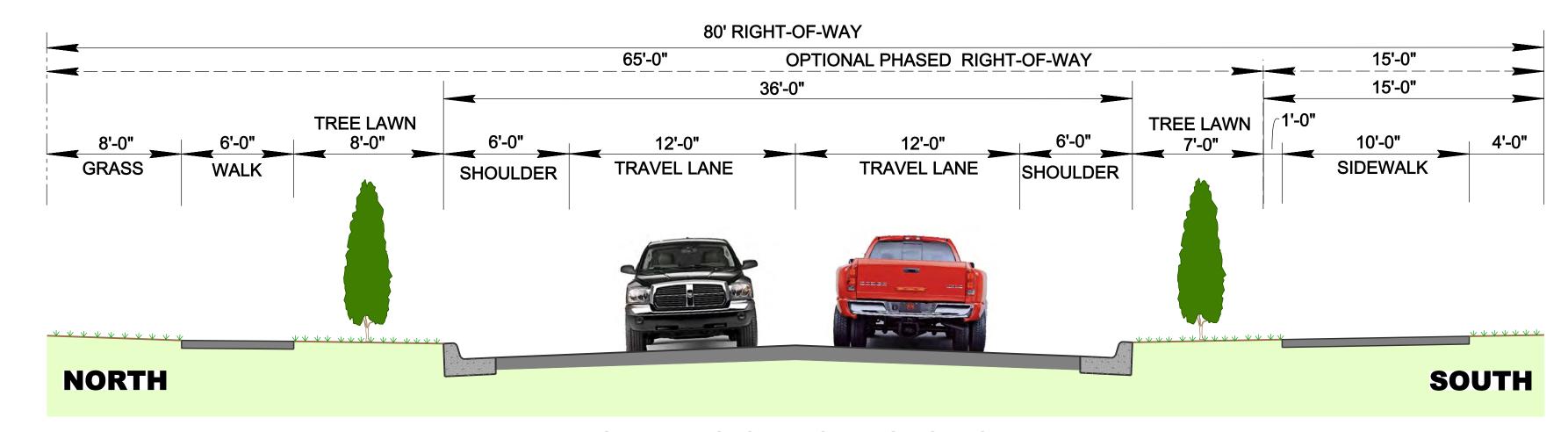
(WALTERSCHEID BLVD TO SOUTH GREELEY HWY) RECOMMENDED STREET SECTIONS



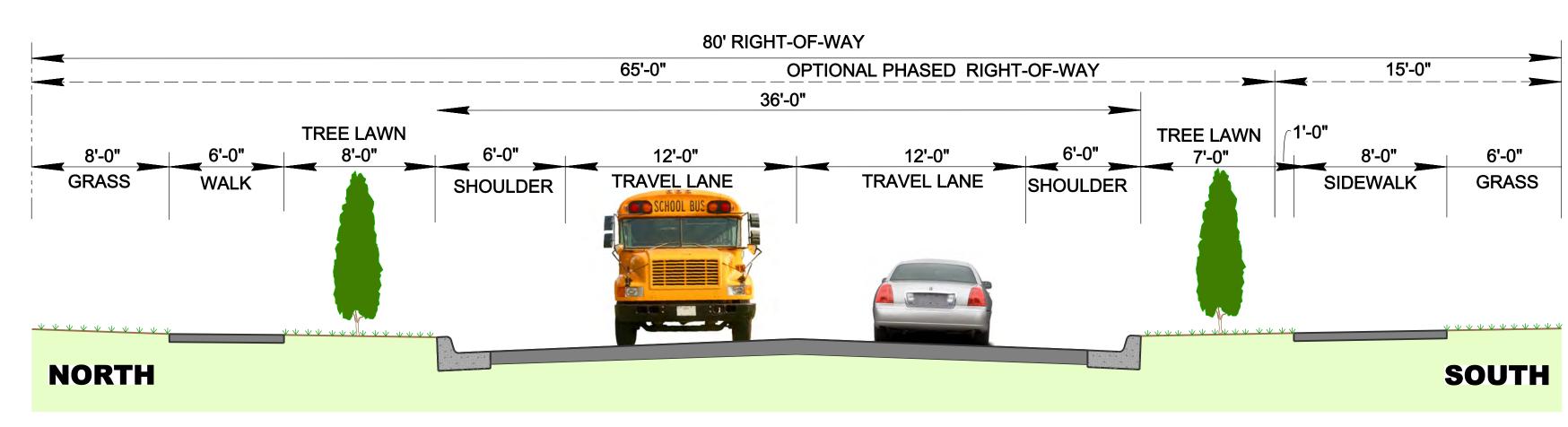
WEST ALLISION ROAD SECTION STA. 442+69.38 to STA. 445+06.99 (AT WALTERSCHEID INTERSECTION) SECTION 1



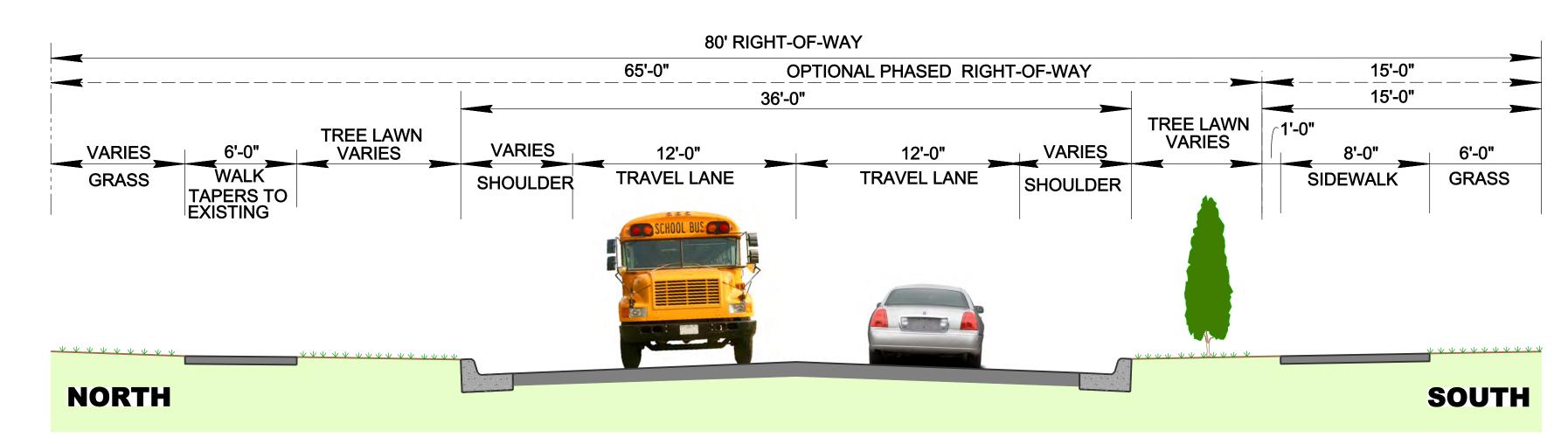
WEST ALLISION ROAD SECTION STA. 445+06.99 to STA. 446+06.99 SECTION 2



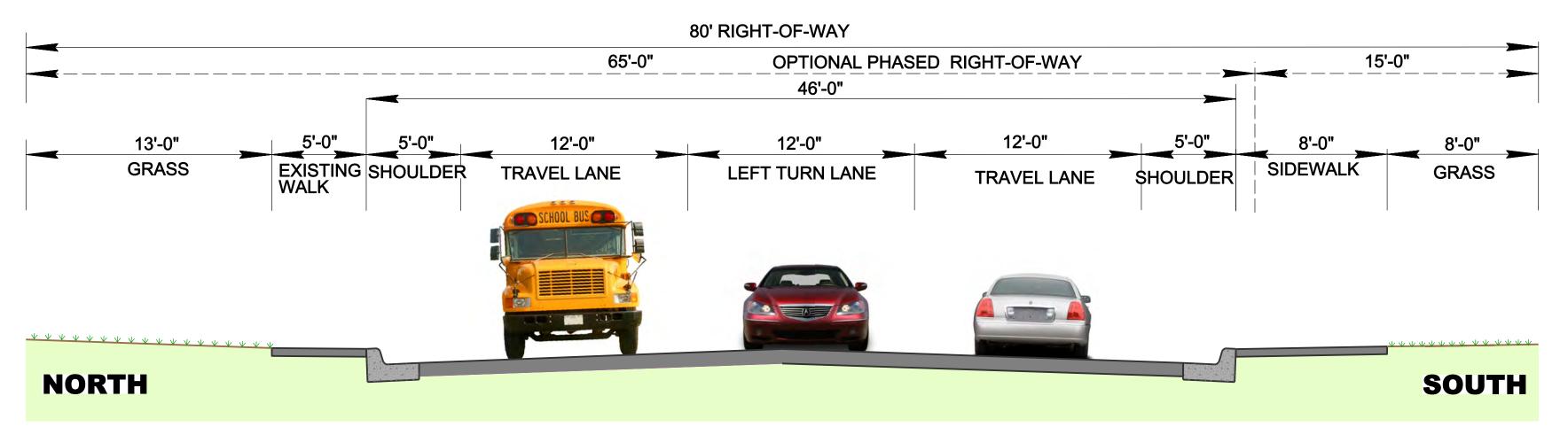
WEST ALLISION ROAD SECTION STA. 446+06.99 to STA. 453+63.80 (AT INTERSECTIONS) SECTION 3



WEST ALLISION ROAD SECTION STA. 453+63.80 to STA. 462+63.91 (AT INTERSECTIONS) SECTION 4



WEST ALLISION ROAD SECTION STA. 462+63.91to STA. 464+34.59 SECTION 5



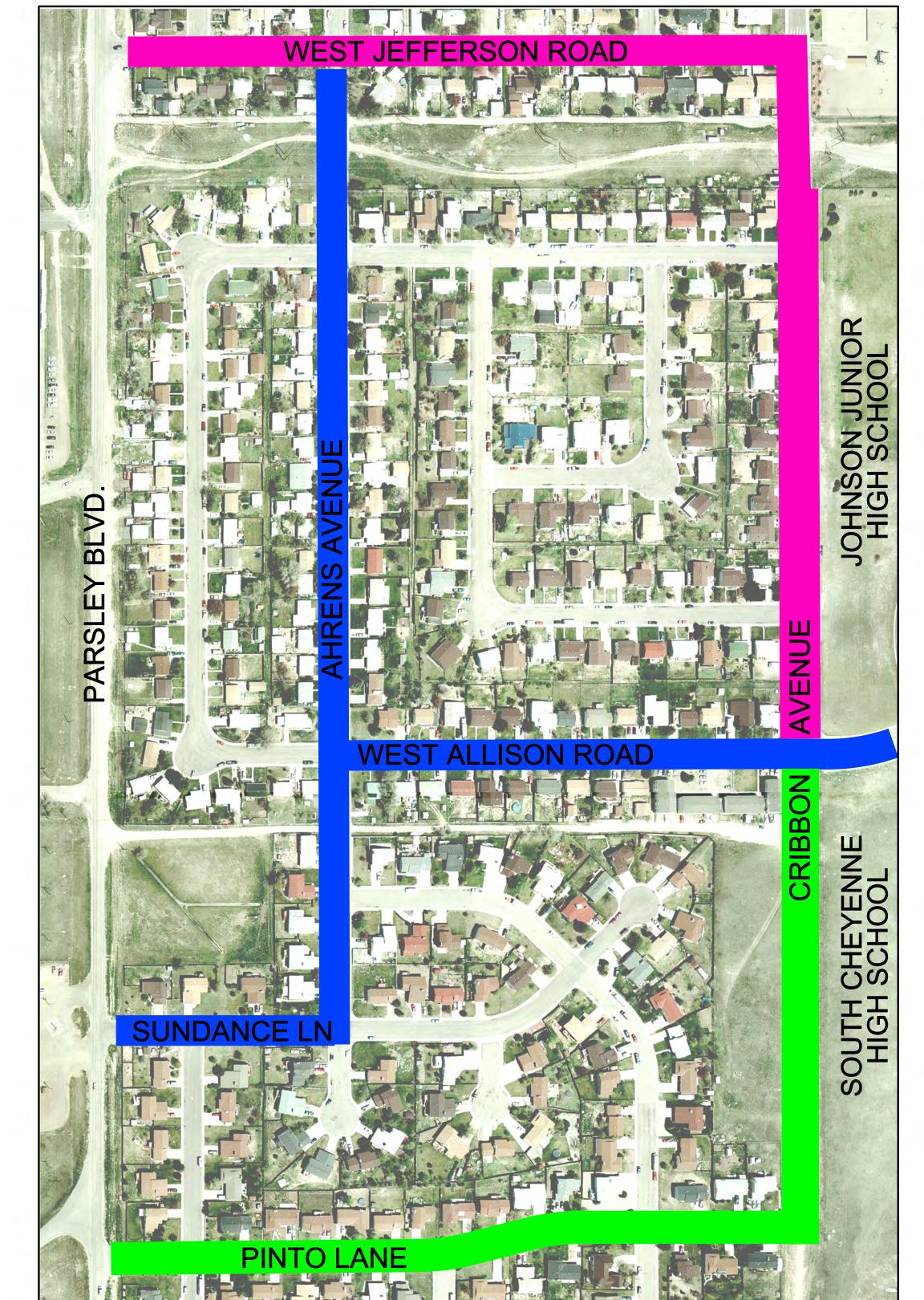
WEST ALLISION ROAD SECTION STA. 464+34.59 to STA. 467+00.00 (AT SOUTH GREELY INTERSECTION) SECTION 6

(SEE RECOMMENDED WALTERSCHEID BOULEVARD TO SOUTH GREELEY HIGHWAY IMPROVEMENTS BOARD FOR PLAN VIEW)





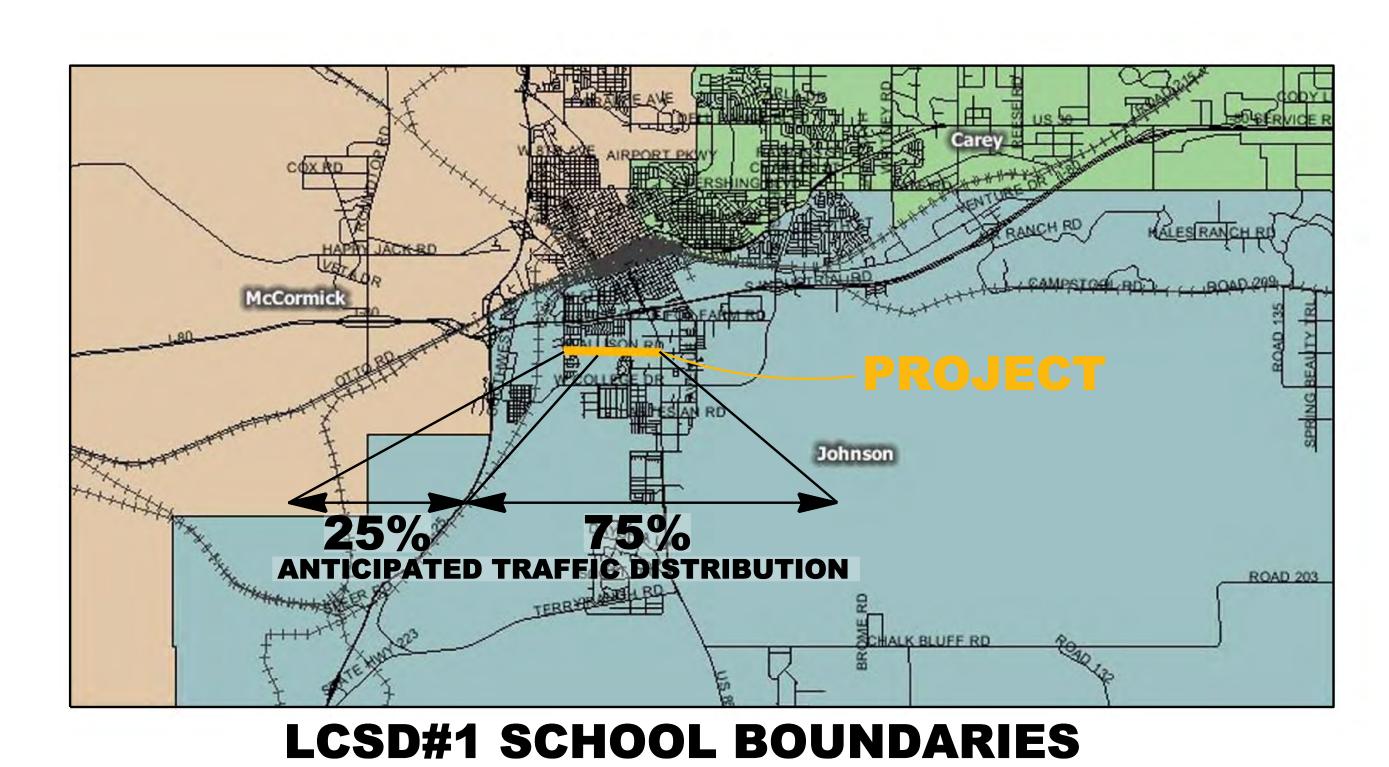
(SNYDER AVENUE TO CRIBBON AVENUE)
RECOMMENDED IMPROVEMENTS



DOHNSON JR. HIGH SCHOOL

TO SAME AND THE SCHOOL

ROUTE CONTINUATION OPTIONS (CONNECTIONS FROM WEST ALLISON RD TO PARSLEY BLVD)



80' RIGHT-OF-WAY

60'-0"

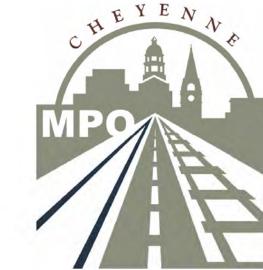
LANDSCAPED ISLAND
12'-0"
11'-0"
12'-0"
11'-0"
12'-0"
11'-0"
12'-0"
11'-0"
12'-0"
11'-0"
11'-0"
12'-0"
11'-0"
11'-0"
10'-0"
TRAVEL LANE
PARKING / BIKE
SIDEWALK

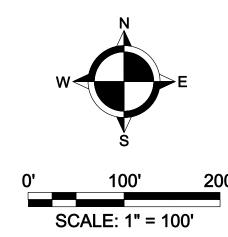
WEST ALLISION ROAD SECTION STA. 406+10 to STA. 413+18 SECTION 7



(SOUTH GREELEY HWY TO SNYDER AVENUE) RECOMMENDED DRAINAGE IMPROVEMENTS









APPENDIX E

Speed Counts

24 HOUR SPEED

Pinto East of Parsley

Data For Site: Pinto East of F Storage Mode: Count Data collected by a Apollo (ver 1.45) Posted speed: 20 Record Interval Length: 15 min

	count	cnood		0011104							
0.00	Count	speed	0.00	count	speed		count	speed		count	speed
0:00			6:00	11	21	12:00	7	28	18:00	23	27
15			15	7	29	15	9	24	15	25	29
30			30	12	27	30	16	25	30	19	30
45			45	9	30	45	12	24	45	16	31
1:00			7:00	14	31	13:00	5	30	19:00	11	26
15			15	19	32	15	7	26	15	7	24
30			30	18	34	30	14	25	30	6	25
45			45	23	33	45	6	30	45	4	22
2:00			8:00	19	27	14:00	5	27	20:00	2	21
15			15	26	24	15	6	28	15	1	27
30			30	17	26	30	5	29	30	3	25
45			45	22	25	45	7	25	45	1	26
3:00			9:00	27	25	15:00	7	26	21:00	1	24
15			15	21	26	15	9	26	15	-	
30			30	28	24	30	11	26	30	2	22
45			45	20	25	45	6	25	45	4	23
4:00	1	22	10:00	12	30	16:00	10	27	22:00	•	23
15	1	27	15	10	24	15	11	26	15	1	26
30	÷		30	15	26	30	16	25	30	2	29
45	2	33	45	12	27	45	18	24	45	-	2)
5:00			11:00	11	25	17:00	29	23	23:00		
15	3	21	15	13	26	15	26	27	15		
30	9	24	30	16	28	30	29	25	30		
45	8	37	45	21	26	45	30	24	45		
	_	٠.	.0	21	20	70	50	24	70		
	24			403			301			128	

24 Hour Total Average Speed

856 26

24 HOUR SPEED

Data For Site :

Allison west of Walterschied

Storage Mode : Count
Data collected by a Apollo (ver 1.45)
Posted speed : 30
Record Interval Length : 15 min

	count	speed		count	speed		count	speed		count	speed
0:00	1	35	6:00	28	33	12:00	63	33	18:00	48	31
15			15	33	32	15	62	32	15	44	32
30			30	39	33	30	51	34	30	41	33
45			45	47	34	45	55	33	45	39	33
1:00			7:00	52	33	13:00	49	32	19:00	44	31
15	1	40	15	55	34	15	54	33	15	42	32
30			30	59	35	30	44	34	30	38	32
45	2	32	45	53	34	45	51	32	45	35	33
2:00			8:00	44	33	14:00	55	34	20:00	35	31
15			15	45	34	15	56	33	15	29	33
30			30	42	33	30	53	32	30	26	32
45			45	39	31	45	49	33	45	22	35
3:00	1	34	9:00	28	32	15:00	49	34	21:00	12	33
15			15	31	33	15	48	35	15	14	35
30			30	27	31	30	51	33	30	9	36
45			45	25	31	45	55	32	45	10	32
4:00	2	33	10:00	32	32	16:00	48	33	22:00	11	33
15	7	32	15	31	33	15	46	34	15	7	34
30	9	31	30	38	32	30	45	35	30	4	33
45	8	30	45	33	32	45	49	33	45	9	32
5:00	16	34	11:00	35	33	17:00	56	34	23:00	3	31
15	14	35	15	33	35	15	57	32	15	1	30
30	15	34	30	31	35	30	59	31	30		
45	17	34	45	37	34	45	51	33	45	1	42
	93			917			1256			524	

24 Hour Total Average Speed 2790 33

24 Hour Speed and Volume Allison between s Greely and Walterscheid

Data For Site : // Storage Mode : Count

Data collected by a Apollo (ver 1.45) Posted speed: 30 Record Interval Length: 15 min

	count	speed		count	speed		count	speed		count	speed
0:00		•	6:00	11	32	12:00	30	31	18:00	19	35
15			15	10	33	15	30	35	15	16	33
30			30	14	31	30	25	36	30	21	32
45	1	32	45	11	33	45	26	35	45	14	33
1:00			7:00	13	35	13:00	29	36	19:00	12	31
15			15	18	34	15	19	33	15	10	32
30	1	31	30	15	33	30	17	34	30	7	31
45			45	16	31	45	29	32	45	15	33
2:00			8:00	22	32	14:00	35	33	20:00	6	28
15			15	42	33	15	28	26	15	7	29
30	1	30	30	39	33	30	29	35	30	5	30
45			45	28	32	45	30	39	45	4	31
3:00			9:00	31	31	15:00	28	33	21:00	5	33
15	1	33	15	25	32	15	22	32	15	5	32
30			30	26	33	30	33	33	30	3	31
45	2	35	45	29	37	45	17	31	45	5	30
4:00			10:00	31	36	16:00	12	34	22:00	6	34
15			15	27	35	15	14	35	15	4	31
30	3	34	30	25	34	30	15	36	30	2	35
45	3	35	45	32	34	45	16	35	45	2	36
5:00	7	36	11:00	27	35	17:00	11	36	23:00	1	32
15	5	35	15	25	33	15	19	33	15	1	33
30	2	36	30	26	34	30	20	32	30	1	34
45	6	38	4 5	28	31	45	21	38	4 5		
	32			571			555			171	

24 Hour Total 1329 Average Speed 33

24 HOUR SPEED

Data For Site: Cribbbon North of Allison Storage Mode: Count Data collected by a Apollo (ver 1.45) Posted speed: 25 Record Interval Length: 15 min

	count	speed		count	spood		count	opood			
0:00		•	0.00		speed	40.00	count	speed		count	speed
	1	32	6:00	10	32	12:00	30	31	18:00	19	35
15	_		15	12	33	15	28	33	15	16	33
30	1	31	30	11	31	30	22	32	30	21	32
45			45	14	33	45	33	34	45	14	33
1:00			7:00	15	35	13:00	17	32	19:00	12	31
15			15	17	34	15	12	33	15	10	32
30			30	12	33	30	14	34	30	7	31
45			45	16	31	45	15	32	45	15	33
2:00			8:00	24	32	14:00	16	33	20:00	6	28
15	1	30	15	23	33	15	11	26	15	7	29
30			30	29	33	30	19	35	30	5	30
45			45	30	32	45	20	39	45	4	31
3:00			9:00	28	31	15:00	21	33	21:00	5	33
15			15	27	32	15	17	32	15	5	32
30			30	23	33	30	15	33	30	3	31
45	1	35	45	28	37	45	18	31	45	5	30
4:00			10:00	18	36	16:00	15	34	22:00	6	34
15			15	14	35	15	14	35	15	4	31
30	2	34	30	11	34	30	19	36	30	2	33
45	1	32	45	13	34	45	17	35	45	2	29
5:00	5	33	11:00	18	35	17:00	29	36	23:00	1	27
15	4	30	15	17	33	15	35	33	15	1	28
30	2	31	30	21	34	30	28	32	30	1	29
45	5	33	45	11	31	45	29	31		1	29
40	,	33	70	11	31	40	29	31	45		
	23			442			494			171	

24 Hour Total Average Speed 1130 32

APPENDIX F

Estimate of Probable Construction Costs

ESTIMATE OF PROBABLE CONSTRUCTION COST

Item	Description	Quantity	Units	Unit Cost	Sub-total
ROW Acquisition					
(residential)	2009 approx. market value land only	1.14	AC	\$40,000	\$45,455
(commercial)	2009 approx. market value land only	0.25	AC	\$95,000	\$23,390
					\$68,845
ROADWAY					
Asphalt Pavement	6" asphalt	3,053	TON	\$62	\$189,283
Road Base	8" thick compacted road base	2,262	CY	\$25	\$56,553
Earthwork	Cut and Fill	5,230	CY	\$5	\$26,150
Concrete Curbing					
(type A)	6" vert. curb with 18" pan	4,426	LF	\$25	\$110,650
Concrete Sidewalk					
(4" thick)	6' wide walk	1,326	SY	\$46	\$60,996
(4" thick)	10' wide walk	1,224	SY	\$46	\$56,304
(4" thick)	8' wide walk	1,162	SY	\$46	\$53,452
Driveways	6" concrete driveways	21	EA	\$8.000	\$168.000
	Improvements to existing driveways	21	EA	\$500	\$10,500
Road Striping	4" white and yellow painted lines	7.210	LF	\$0.40	\$2,884
.ouu o.i.p.i.g	18" stop bars	6	EA	\$100	\$600
	symbols	6	EA	\$150	\$900
	Crosswalk	6	EA	\$150	\$900
Signage	school crossing etc.	2	EA	\$200	\$400
99-	Stop signs at north/south connector	2	EA	\$200	\$400
	speed limit	4	EA	\$200	\$800
JTILITIES					\$738,772
Julia Ties Julia Ties					
(electric)	relocate signal poles	4	EA	\$5,000	\$20,000
(electric)	relocate electric poles	5	EA	\$5,000	\$25,000
Jility Improvements					
(water)	replace exist. 6" w/ new 8" water main	2,375	LF	\$70	\$166,250
(water)	8" water main in north/south connector	120	LF	\$70	\$8,400
(water services)	1" service lines	21	EA	\$1,000	\$21,000
(sewer)	replace exist. 6" w/ new 8" sewer main	1,475	LF	\$50	\$73,750
(sewer services)	4" service lines	21	EA	\$1,000	\$21,000
Street Lighting	30' tall fiberglass pole w/ 135 watt LPS light fixture	24	EA	\$2,500	\$60,000
COUNDADOUT	Positive (Control			0050 000	\$395,400
ROUNDABOUT	Roundabout Construction	1	EA	\$250,000	\$250,000
	Roundabout Landscaping	1	EA	\$16,000	\$16,000
					\$266,000

ESTIMATE OF PROBABLE CONSTRUCTION COST

DRAINAGE IMP	ROVEMENTS					
	w culverts)	18" HDPE corrugated pipe	60	LF	\$21.50	\$1,290
	new inlets)	area inlets, curb inlets, etc.	4	EA	\$3,000	\$12,000
,	torm pipe)	30" RCP	100	LF	\$138	\$13,800
	torm pipe)	36" RCP	1,260	LF	\$120	\$151,200
(3	(FES)	30" FES	1,200	EA	\$1,215	\$1,215
	gasket	36"	17	EA	\$16	\$272
ctorm	manholes	48" manhole	5	EA	\$2,500	\$12,500
5101111			1	LS	\$20,000	\$20,000
	(grading)	misc. grading	ı	LS	\$20,000	\$20,000
						\$212,277
IRRIGATION A						
Irrigation	(SCW&S)	1" water tap	1	EA	\$1,295	\$1,295
		1" water tap service development fee	1	EA	\$7,830	\$7,830
		1" water meter for irrigation	1	EA	\$350	\$350
		Meter Pit	1	EA	\$2,900	\$2,900
		1" pvc lines for new landscaping	1,500	LF	\$1.5	\$2,250
Landscaping						
	(trees)	15 to 30 gal	10	EA	\$250	\$2,500
	(shrubs)	3, 5 & 7 gal	100	EA	\$20	\$2,000
	(mulch)	3"-4" thick	30	CY	\$40	\$1,200
	(/	seeding	2.760	SY	\$0.35	\$966
		2" landscape rock	410	CY	\$100	\$41,000
		Irrigation clock/controller	1	EA	\$1,600	\$1,600
						\$63,891
IMPROVEMENT:	S IN FRONT O	F JOHNSON JUNIOR HIGH				φ03,691
Concrete Curbing	9					
	(type A) 6'	vert. curb with 18" pan - infall for medians and bumpouts on Allison	1,130	LF	\$25	\$28,250
		6" vert. curb with 18" pan - infall for Cribbon Driveway	368	LF	\$25	\$9,200
		6" vert. curb with 18" pan - infall for Cribbon medians	644	LF	\$25	\$16,100
		splash guard	900	LF	\$25	\$22,500
Asphalt Pavemer	nt	6" asphalt for Cribbon Driveway	466	TON	\$62	\$28,892
Road Base		8" thick compacted road base for Cribbon Driveway	124	CY	\$25	\$3,100
Road Striping		18" stop bars	2	EA	\$100	\$200
		4" white and yellow painted lines on Allison	2.189	LF	\$0.40	\$876
		4" white and yellow painted lines on Cribbon	3,953	LF	\$0.40	\$1,581
		symbols	2	EA	\$150	\$300
		Crosswalk	2	EA	\$150	\$300
Signage		Stop Signs on side streets of Pinto Lane	5	EA	\$200	\$1,000
Landscape		Median landscape rock	120	CY	\$100	\$12,000
Lanuscape		Medain xeriscape landscaping on Allison	1	SF	\$6,000	\$6,000
		Medain xeriscape landscaping on Cribbon	1	SF SF	\$2,000	\$2,000
					• ,	
						\$132,299
					Subtotal	\$1,877,483
Contingency			20	%		\$375,497
Bond & Insurance	е		2	%		\$37,550
General Condition	ns (Traffic Con	trol, Mobilization, etc.)	10	%		\$187,748
Total Construction	n Cost (Prelimi	nary)			Total	\$2,478,278
	•					
Engineering Cost	t			8 %		\$198,262.23
Survey Cost				2 %		\$49,565.56
Construction Man	nagement Cost			10 %		\$247,827.78
					Total	\$2,973,933

APPENDIX G

Additional Information

Counter:

Counted By: James & Lynn Weather:Clear

Other:

File Name: JEFFER~1

Site Code : 00000000

Start Date : 2/10/2009

Page No : 1

Otner:			1		Groups Printed- Vehicles V Cribbon W Jefferson) .	I						
			Cribbo						Gioups	Fillitet	i- A CIII			n	-	 ;	W	Jeffers	on		
	I		om No			D: 1		om E			5: T		om So			T		om We			1.1
Start Time	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr	Left	Ped s	App. Total	Rig ht	Thr	Left	Ped	App. Total	Rig ht	Thr	Left	Ped	App. Total	Int. Total
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
07:00 AM	2	13	0	0	15	0	0	0	1	1	0	18	21	0	39	11	0	0	0	11	66 54
07:15 AM 07:30 AM	2 1	11 6	0	0	13 7	0	0	0 0	0 0	0	0	19 23	12 12	0	31 35	6 8	0	4 6	0	14	5 4 56
07:45 AM	i	10	Ö	0	11	0	Ö	ő	Ö	o	Ö	17	9	0	26	2	Ö	9	Ö	11	48
Total	6	40	0	. 0	[46]	0	0	0	1	1	0	77	54	0	131	27	0	19	0	46	224
00.00 444		_	^	0	0.1	•	0	_		ا م	^	40	7	0	22	4	0	40	0	441	40
08:00 AM 08:15 AM	1 1	5 5	0 0	0 0	6 6	0	0	0	0	0	0	16 15	7 21	0 0	23 36	4 5	0	10 4	0 0	-1 4	43 51
08:30 AM	1	5	0	Ö	6	0	Ő	Ö	0	ŏ	0	4	9	0	13	7	ő	4	Ö	11	30
08:45 AM	0	2	0	0	2	0	0	0	0	0	0	3	4	0	7	7	0	0	0	7	16
Total	3	17	0	0	20	0	0	0	0	0	0	38	41	0	79	23	0	18	0	41	140
BREAK																					
11:00 AM	0	5	0	0	5	0	0	0	0	0	0	1	4	0	5	8	0	2	0	10	20
11:15 AM	2	4	Ö	2	8	Ö	ő	Ö	Ö	ŏ	Ö	10	6	Ö	16	5	Ö	4	Ō	9	33
11:30 AM	0	4	0	1	5	0	0	0	0	0	0	6	5	0	11	2	0	0	0	2	18
11:45 AM	0	5	0	1	6	0	0	0	0	0	0	3	1	0	4	4	0	3	0	7	17
Total	2	18	0	4	24	0	0	0	0	0	0	20	16	0	36	19	0	9	0	28	88
12:00 PM	2	4	0	0	6	0	0	0	0	0	0	6	8	0	14	5	0	2	0	7	27
12:15 PM 12:30 PM	0 2	1 4	0	0 0	1 6	0 0	0 0	0	0 0	0	0 0	5 6	5 6	0	10 12	2 7	0	1 1	0 0	3 8	14 26
12:45 PM	0	4	0	0	4	0	0	Ö	0	ő	0	6	5	Ö	11	6	0	4	0	10	25
Total	4	13	0	0	17	0	0	0	0	0	0	23	24	0	47	20	0	8	0	28	92
BREAK																					
02:00 PM	2	6	0	0	8	0	0	0	0	0	0	10	4	0	14	9	0	1	0	10	32
02:15 PM	1	6	Ö	Ö	7	Ö	0	Ö	Ö	ŏ	ő	2	2	Ö	4	7	Ö	2	Ö	9	20
02:30 PM	1	7	0	-0	8	0	0	0	5	5	0	26	13	0	39	9	0	1	2	12	64
02:45 PM	2 6	12 31	0	0	14 37	0	0	0	<u>1</u>	1 6	0	7 45	23	0	11 68	7 32	0	<u>2</u> 6	<u>1</u>	10 41	36 152
Total	0	31	U	U	37	U	U	U	O	0	U	40	23	U	00	32	U	O	3	411	132
03:00 PM	3	7	0	0	10	0	0	0	3	3	0	11	6	0		. 9	0	4	4	17	47
03:15 PM	4	11	0	_0_	15	0	0	0	30	30	0	16	15	0	31	14	0	3	2	19	95
03:30 PM 03:45 PM	8 3	7	0	0 0	15 15	0 0	0 0	0	10 11	10 11	0	10 8	9 7	0 0	19 15	14 15	, 0	5 4	0 5	19 - 24	63 65
Total	<u>3</u> 18	12 37	0	0	55	0	0	0	54	54	0	45	37	0	82	52	, 0	16	11	79	270
			_	_		_	_	_	-	- 1	_			_						,	
04:00 PM	6	16	0	1	23	0	0	0	4	4	0	12	17	1	30	6	0	2	0	8	65
04:15 PM 04:30 PM	4	11 9	0	2	17	0	0 0	0	1 0	1 0	0	8 8	6 9	0	14 17	9 10	0	4 7	0	13 17	45 47
04:35 PM	4 2	8	0	0	10	0	0	0	2	2	0	5	9	0	14	13	0	ó	0	13	39
Total	16	44	0	3	63	0	0	0	7	7	0	33	41	1	75	38	0	13	0	51	196
05:00 PM	5	7	0	0	12	0	0	0	0	o l	0	4	6	0	10	22	0	1	0~	23	45
05:15 PM	5	7	0	0	12	0	0	0	Ö	0	0	12	6	0	18	12	Ö	3	4	19	49
05:30 PM	1	7	0	0	8	0	0	0	0	o	0	7	5	0	12	14	0	2	6	22	42
05:45 PM	3	6	0	0	9	0	0	0	0	0	0	5	5	0_	10	11	0	2	3	16	35
Total	14	27	0	0	41	0	0	0	0	0	0	28	22	0	50	59	0	8	13	80	171
Grand Total	69	227	0	7	303	0	0	0	68	68	0	309	258	1	568	270	0	97	27	394	1333
Apprch %	22.8	74.9	0.0	2.3		0.0	0.0	0.0	100.		0.0	54.4	45.4	0.2		68.5	0.0	24.6	6.9		
Total %		17.0	0.0	0.5	22.7	0.0	0.0	0.0	0 5,1	5.1	0.0		19.4	0.1	42 f	20.3	0.0	7.3	2.0	29.6	
10(01 70	0.2	11.0	0.0	0.0		0.0	0.0	0.0	٠, ١	J. 1	0.0	20.2		J. 1	12.0		5.0	0		20.0	

Counter:

Counted By: James & Nancy

Weather: Clear

Other:Walterscheid N of 5th St Closed

File Name: WALTER~1

Site Code : 00021209 Start Date : 2/12/2009

Demo No 11

Page No : 1
Groups Printed- Vehicles - Axles 2+

			ersche rem No	id Blvd				Ilison rom E	Rd	100 10			ersche om So	id Blvd				llison I			
	Dia		OHI INC		A	Dia		TOTIL		Λ	Dia		OHI OU	, , , ,	۸	Dia	Thr	OIII VV		Λ ¬ ¬	Int
Start Time	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	u	Left	Ped s	App. Total	Int. Total
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
11:00 AM	4	17	4	0	25	2	3	4	1	10	5	11	5	0	21	2	8	12	0	22	78
11:15 AM	2	16	5	0	23	4	6	2	1	13	4	15	5	0	24	3	5	4	0	12	72
11:30 AM	1	15	1	1	18	2	11	7	2	22	4	13	4	0	21	5	10	8	0	23	84
11:45 AM	4	17	7	0	28	0	10	3	0	13	5	14	4	0	23	4	11	4	0	19	83
Total	11	65	17	1	94	8	30	16	4	58	18	53	18	0	89	14	34	28	0	76	317
12:00 PM	9	30	1	0	40	6	10	6	1	23	7	31	7	0	45	6	17	6	2	31	139
12:15 PM	6	28	2	2	38	5	18	2	0	25	1	14	5	0	20	3	5	1	0	9	92
12:30 PM	4	21	3	0	28	3	13	8	0	24	5	20	2	0	27	5	8	2	0	15	94
12:45 PM	9	21	1	0	31	3	8	7	2	20	7	32	6	1	46	3	6	6	0	15	112
Total	28	100	7	2	137	17	49	23	3	92	20	97	20	1	138	17	36	15	2	70	437
BREAK																					
02:00 PM	12	13	3	0	28	1	9	1	0	11	2	11	6	0	19	3	11	8	0	22	80
02:15 PM	13	21	9	1	44	6	21	5	0	32	3	9	9	0	.21	2	6	4	0	12	109
02:30 PM	10	20	4	1	35	3	16	7	0	26	4	10	11	0	25	17	26	25	2	70	156
02:45 PM	6	21	3	0	30	8	13	9	0	30	7	15	4	2	28	8	17	8	0	33	121
Total	41	75	19	2	137	18	59	22	0	99	16	45	30	2	93	30	60	45	2	137	466
03:00 PM	5	33	1	0	39	7	15	7	0	29	6	20	5	0	31	13	14	6	2	35	134
03:15 PM	10	18	2	0	30	4	22	7	0	33	13	30	16	0	59	12	17	4	0	33	155
03:30 PM	13	23	5	0	41	3	18	10	0	31	8	24	7	0	39	9	36	21	1	67	178
Grand Total	108	314	51	5	478	57	193	85	7	342	81	269	96	3	449	95	197	119	7	418	1687
Apprch %	22.6	65.7	10.7	1.0		16.7	56.4	24.9	2.0		18.0	59.9	21.4	0.7		22.7	47.1	28.5	1.7		
Total %	6.4	18.6	3.0	0.3	28.3	3.4	11.4	5.0	0.4	20.3	4.8	15.9	5.7	0.7	26.6	5.6	11.7	26.5 7.1	0.4	24.8	
						'					'					'					

Counter Board: 64 Counted By: carrol Weather: clear File Name : ALLISO~1 Site Code : 00000238 Start Date : 4/24/2007 Page No : 1

Other: cheyenne counts 2007

Groups Printed- Unshifted - Bank 1 - Bank 2

							Gro	oups P	rinted-	Unshifte	ed - Ba							V			
			S. Hwy					Ilison F		ł			S. Hwy					Ilison I			
		Fṛ	om No	<u>rth</u>				rom Ea	ast _				<u>om Sọ</u>	uth				rom W	est		
Start Time	Rt	Thr	Left	Ped	App.	Rt	Thr	Left	Ped	App.	Rt	Thr	Left	Ped	App.	Rt	Thr	Left	Ped	App.	Int.
	10	u			Total		u			Total		u			Total	1.0	4 O		1.0	Total	Total
Factor 07:00 AM	1.0	1.0 49	1.0	1.0	60	1.0 2	1.0	1.0 1	1.0 0	8	1.0 1	1.0 109	1.0 8	1.0	118	1.0	1.0 1	1.0 15	1.01	19	205
07:00 AM 07:15 AM	6	58	2	0	66	5	4	1	0	10	2	105	9	0	116	7	6	19	0	32	224
07:13 AM	4	60	2	0	66	4	6	0	0	10	3	158	4	0	165	9	8	13	0	30	271
07:30 AM 07:45 AM	8	104	1	0	113	6	2	1	0	9	3	193	6	0	202	10	2	- 6	0	18	342
Total	25	271	9	0	305	17	17	3	0	37	9	565	27	0	601	29	17	53	0,	99	1042
Total	20	211	3	U	303	17	17	3	U	3/	9	303	21	U	0011	23	17	55	وں.	331	1042
08:00 AM	5	89	5	0	99	3	3	3	0	9	4	116	3	0	123	1	3	10	0	14	245
08:15 AM	6	88	6	0	100	3	3	3	0	9	4	114	6	0	124	6	3	2	2	、 13	246
08:30 AM	0	78	4	0	82	5	7	1	0	13	5	121	2	0	128	5	4	2	0	11	234
08:45 AM	2	76	6	0	84	2	3	1	0	6	3	114	2	0	119	3	1	7	0	11	220
Total	13	331	21	0	365	13	16	8	0	37	16	465	13	0	494	15	11	21	2	49	945
BREAK																					
					1					_ 1					1		_		_	1	
11:00 AM	6	117	0	0	123	3	3	2	1	9	2	141	8	0	151	9	8	22	0	39	322
11:15 AM	5	129	3	0	137	1	2	3	0	6	1	117	9	0	127	10	2	17	0	29	299
11:30 AM	8	117	6	0	131	2	3	5	0	10	0	128	3	0	131	8	6	15	0	29	301
11:45 AM	14	151	4	0	169	9	7	2		19	2	154	9	0	165	9	4	8	0	21	374
Total	33	514	13	0	560	15	15	12	2	44	5	540	29	0	574	36	20	62	0	118	1296
12:00 PM	13	166	8	0	187	9	5	2	0	16	6	145	22	0	173	13	8	14	3	38	414
12:15 PM	10	160	12	0	182	3	5	0	0	8	6	183	16	0	205	4	7	9	0	20	415
12:30 PM	13	117	5	0	135	9	6	2	0	17	3	148	12	0	163	7	3	17	0	27	342
12:45 PM	7	132	5	0	144	2	2	4	0	8	1	152	6	0	159	10	1	9	0	20	331
Total	43	575	30	0	648	23	18	8	0	49	16	628	56	0	700	34	19	49	3	105	1502
BREAK																					
03:00 PM	5	127	4	0	136	9	0	1	0	10	3	119	9	0	131	4	2	8	0	14	291
03:15 PM	5	137	11	Ŏ	153	4	ŏ	4	ŏ	8	1	111	4	Ö	116	6	0	9	Ö	15	292
03:30 PM	7	160	3	ō	170	4	3	1	Ö	8	1	135	8	Ö	144	8	3	15	Ō	26	348
03:45 PM	8	158	3	Ō	169	2	4	2	0	8	1	144	5	0	150	7	2	7	0	16	343
Total	25	582	21	0	628	19	7	8	0	34	6	509	26	0	541	25	7	39	0	71	1274
04:00 PM	9	156	11	0	176	3	1	3	0	7	2	137	4	0	143	0	4	13	1	18	344
04:15 PM	8	175	10	0	193	1	2	4	0	7	1	108	9	ŏ	118	10	3	9	ó	22	340
04:30 PM	9	133	6	3	151	6	2	1	ŏ	9	2	135	13	ő	150	6	3	12	ő	21	331
04:45 PM	12	159	6	4	181	8	3	3	ŏ	14	0	144	8	ő	152	11	2	4	1	18	365
Total	38	623	33	7	701	18	8	11	0	37	5	524	34	0	563	27	12	38	2	79	1380
05:00 PM	5	207	13	0	225	5	3	3	0	11	2	130	14	0	146	5	6	16	0	27	409
05:00 FM	12	204	12	0	228	7	3	3	0	13	2	146	10	0	158	~ 8	0	9	0	17	416
05:30 PM	14	158	10	0	182	4	5	1	0	10	. 2	119	10	0	131	8	3	10	0	21	344
05:45 PM	11	116	3	0	130	3	5	Ó	0	8	. 2	107	7	0	115	9	1	10	1	21	274
Total	42	685	38	0	765	19	16	7	0	42	7	502	41	0	550	30	10	45	1	86	1443
Grand	0.46	358	405	_	2070				-	ا مور	0.1	373	000		ا مود	400	0.0	007		007	2000
Total	219	1	165	7	3972	124	97	57	2	280	64	3	226	0	4023	196	96	307	8	607	8882
Apprch %	5.5	90.2	4.2	0.2		44.3	34.6	20.4	0.7		1.6	92.8	5.6	0.0		32.3	15.8	50.6	1.3		
Total %	2.5	40.3	1.9	0.1	44.7	1.4	1.1	0.6	0.0	3.2	0.7	42.0	2.5	0.0	45.3	2.2	1.1	3.5	0.1	6.8	

Counter: Counted By: Ayres Associates Weather:

Other:

File Name: SWALTE~1 Site Code : 00052008

Start Date : 5/20/2008 Page No : 1

						G	Froups F	Printed-	All Vehi	cles				V	/		
	W	altersch	neid Blv	/d		Alliso	n Rd		W	altersch	neid Blv	⁄d		Alliso	n Rd		
		From	North			From	East			From	South			From	West		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int.
	_				_												Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
07:00 AM	39	13	3	<i>5</i> 5 0	1	18	_1_	20 0	5	25	32	0	16	26	19	0	198
_07:15 AM	39	10	0	49 0	4	8	0	12 0	_ 4	22	18	0	13	21	21	0	160
07:30 AM	6	8	_ 5	19 0	4	6	3	13 0	6	33	10	0	7	13	7	0	108
07:45 AM	4	16	2	22 0	2	15	6	23 0	3	39	6	0	7	8	7	0	115
Total	88	47	10	145 0	11	47	10	(8 O	18	119	66	2030	43	68	54	1650	581
	_	₹	£4.			_	_		1 =					_	_	- 1	
08:00 AM	2	19	3	24 0	2	9	2	13 0	3	22	1	0	1	7	8	0	79
08:15 AM	2	15		23 0	4	11	6	220	. 4	21	3	0	7	9	3	0	91
08:30 AM	5	16	_	23 0	2	2	7	11 0	3	16	1	0	4	9	8	0	75
08:45 AM	5	13	3	21 0	3	5	1_	9 0	0	13	6	0	1	7	6	0	63_
Total	14	63	14	91 0	11	27	16	0	10	72	11	0	13	32	257	0	308
*** DDE AV ***																	
*** BREAK ***																	
03:00 PM	6	22	3	0	l =	18	4	0	3	19	8	0	7	12	12	1	120
03:00 PM	5	30	ა 3	0	5 5	13	4	2	17	19	8	0	3	12	7	6	120
03:30 PM	9	32	ა 11	0	4	14	5 2	0	8	20	2	0	8	25	8	0	143
03:45 PM	5	23	6	0	7	18	4	0	6	19	6	0	7	12	14	0	127
Total	25	107	23	0	21	63	15	2	34	72	24	0	25	61	41	1	514
Total	23	107	23	U	21	03	13	~	J 4	12	4	U	23	01	71	'	314
04:00 PM	10	37	8	0	4	15	6	0	l 3	18	5	0	l 3	15	7	0	131
04:15 PM	5	30	7	Ö	11	13	6	Ö	7	28	5	Ō	2	5	4	ō	123
04:30 PM	5	35	4	3	6	12	_	22 1	3	15	4	1	2	12	7	1	116
04:45 PM	4	40	5	1	7	14	8	29 0	4	21	6	1	3	12	3	18 0	129
Total	24	142	24	194 4	28	54	25	1	17	82	20	1212	10	44	21	1	499
				***	'				•			, 6, 1					
05:00 PM	5	58	6	0	5	16	6	270	7	21	6	0	4	11	4	19 0	149
05:15 PM	7	52	5	0	5	13	12	300	11	16	5	1	4	12	5	21 0	148
05:30 PM	5	43	4	0	9	14	6	z9 1	8	31	6	0	5	12	2	19 0	146
05:45 PM	2	30	6	0	10	11	6	270	2	19	4	0	6	13	7	26 0	116
Total	19	183	21	22000	29	54	30	1	28	-87	21	1371	19	48	18	0	559
												-					
Grand Total	170	542	92	4	100	245	96	4	107	432	142	3	110	253	159	2	2461
Apprch %	21.0	67.1	11.4	0.5	22.5	55.1	21.6	0.9	15.6	63.2	20.8	0.4	21.0	48.3	30.3	0.4	
Total %	6.9	22.0	3.7	0.2	4.1	10.0	3.9	0.2	4.3	17.6	5.8	0.1	4.5	10.3	6.5	0.1	

Counter:

Counted By:Lynn & James

Weather:Clear

Other:5th & Deming Closed

File Name: DESMET~1

Site Code : 00022609

Start Date : 2/26/2009

Page No : 1

Other oth &	Demi	ing Ci	osea			_	D 1 1	V. Line		0.			Page	3 110	. 1	
			Desmet	<u> </u>		Groups		d-Vehicle on Rd	s - Axeis T	2+			Allie	√ <i>l</i> on Rd		
			rom Nor					n East		From	South			West		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Factor	1.0	1.0	1.0	1.0	- 10101	1.0	1.0	1.0	- i Otai	1.0	10101	1.0	1.0	1.0		
07:00 AM	3	0	1	0	4	1	81	0	82	0	0	52	1	-0	53	139
07:15 AM	3	0	2	0	5	0	60	0	60	0	0	61	2	0	63	128
07:30 AM	1	0	4	0	5	2	16	0	18	0	0	24	1	0	25	48
07:45 AM	1	0	8	. 0	9	2	14	0	16	0	0	11	1_	0	12	37
Total	8	0	15	0	23	5	171	0	176	0	0	148	5	0	153	352
MA 00:80	0	0	4	0	4	7	12	0	19	0	0	11	0	0	11	34
08:15 AM	0	0	8	0	8	4	18	0	22	0	0	17	0	0	17	47
08:30 AM	0	0	6	0	6	3	9	0	12	0	0	12	0	0	12	30
08:45 AM Total	<u>1</u> 1	0	5 23	0 0	6 24	8 22	12 51	0	20 73	0	0	12 52	1 1	0	13 53	<u>39</u> 150
		U	23	U	24	22	31	U	73 [U	١٠	52	ı	U	33	100
BREAK																
11:00 AM	2	0	1	0	3	4	14	0	18	0	0	14	0	0	14	35
11:15 AM	0	0	3	0	3	2	11	0	13	0	0	20	0	0	20	36
11:30 AM	0	0	2	0	2	4	13	0	17	0	0	18	1	0	19	38
11:45 AM	5	0	9	0	14	4	15	0	19	0	0	18	1	0	19	52
Total	7	0	15	0	22	14	53	0	67	0	0	70	2	0	72	161
12:00 PM	2	0	6	0	8	10	22	0	32	0	0	45	3	0	48	88
12:15 PM	2	0	1	0	3	6	27	0	33	0	0	13	0	0	13	49
12:30 PM	3	0	1	0	4	3	19	0	22	0	0	13	0	0	13	39
12:45 PM	1	0	4	0	5	6	19	0	25	0	0	14	0	0	14	44
Total	8	0	12	0	20	25	87	0	112	0	0	85	3	0	88	220
BREAK																
02:00 PM	1	0	4	0	5	4	23	0	27	0	0	12	1	0	13	45
02:15 PM	1	0	3	0	4	3	45	0	48	0	0	17	1	0	18	70
02:30 PM	1	0	3	0	4	6	20	0	26	0	0	77	2	0	79	109
02:45 PM	3	0	2	0	5	2	18	0	20	0	0	.28	0	0	28	53
Total	6	0	12	0	18	15	106	0	121	0	0	134	4	0	138	277
03:00 PM	2	0	7	0	9	5	17	0	22	0	0	21	3	0	24	55 72
03:15 PM	2	0	9	0	11	10	16	0	26	0	0	32	3	0	35	72
03:30 PM	1	0	4	0	5	9	25	0	34	0	0	58	0	0	58	97
03:45 PM	3	0	8	0	11	10	18	0	28	0	0	26	3	0	29	68
Total	8	0	28	0	36	34	76	0	110	0	0	137	9	0	146	292
04:00 PM	0	0	4	0	4	7	23	0	30	0	0	24	2	0	26	60 77
04:15 PM	1	0	7	0	8	7	28	0	35	_ 0	0	31	3	0_		
04:30 PM	0	0	4	0	4	11	24	0	35	0	0	19	1	1	21	60
04:45 PM	2	0	6	1	9	11	25	0	36	0	0	27	3	0	30	75
Total	3	0	21	1	25	36	100	0	136	0	0	101	9	1	111	272
05:00 PM	0	0	4	0	4	9	23	0	32	0	0	27	2	0	29	65
05:15 PM	0	0	7	0	7	6	37	0	43	- 0	0	30	1	0	31	81
05:30 PM	1	0	5	0	6	5	24	0	29	0	0	22	1	0	23	58
05:45 PM Total	12	0	<u>5</u> 21	0	6 23	24	20 104	0	24 128	0	0	21 100	<u>1</u> 5	0	22 105	52 256
								•		'				_		
Grand Total	43	0	147	1	191		748	0	923	0.0	0	827 95.5	38 4.4	1 0.1	866	1980
Apprch %	22.5 2.2	0.0 0.0	77.0 7.4	0.5	9.6	19.0 8.8	81.0 37.8	0.0 0.0	46.6		0.0	1	1.9		43.7	
Total %	2.2	0.0	7.4	0.1	9.6	0.0	37.0	0.0	40.0	1 0.0	0.0	41.0	1.9	0.1	43.7	I

Groups Printed- Vehicles

Counter:

Counted By:James Lynn Weather:Clear

Other:

File Name: CRIBBO~1

Site Code : 00021009

Start Date : 2/10/2009

Page_yNo :1

			0.11.							s Printed								Allinar			
			Cribbo		l		_	Allisor		ļ	Sc			nstruct	ion			Allisor		1	
			om No					rom Ea					om So					om W			
Start Time	Rig	Thr	Left	Ped	App.	Rig	Thr	Left	Ped	App.	Rig	Thr	Left	Ped	App.	Rig	Thr	Left	Ped	App.	Int.
Start Time	ht	u l	ren	s	Total	ht	u	Feir	s	Total	ht	u	Lon	s	Total	ht	u	Lon	s	Total	Total
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
07:00 AM	1	0	30	1	32	42	10	1	1	^c 54	1	0	1	1	3	0	23	2	6	31	120
07:15 AM	1	1	20	ò	22	29	7	3	0	39	Ö	Ö	Ö	Ö	o l	Õ	11	1	1	13	74
										1				0		0	11	3	i	15	32
07:30 AM	1	0	8	0	9	1	6	0	0	7	1	0	0		1	-					
07:45 AM	2	0	4	0	6	7	4	1_	0	12	0	0	0	0	0	3_	5	3	0	11	29
Total	5	1	62	1	69	79	27	5	1	112	2	0	1	1	4	3	50	9	8	70	255
																				,	
08:00 AM	4	0	6	0	10	2	2	4	1	9	1	0	0	0	1	1	5	8	6	20	40
08:15 AM	0	0	2	0	2	3	2	1	0	6	0	1	0	0	1	0	3	6	3	12	21
08:30 AM	4	0	5	0	9	0	4	0	0	4	1	0	1	0	2	1	5	1	0	7	22
08:45 AM	0	2	4	Ö	6	2	3	2	1	8	1	0	Ó	0	1	0	6	0	0	6	21
Total	8	<u>-</u> 2	17	0	27	7	11	7	2	27	3	1	1	0	5	2	19	15	9	45	104
Total	U	_	.,	U	21	,		,	_	211	J		•	Ū	0 1	_		10	Ü	.0	,,,,
BREAK																					
11:00 AM	1	0	3	0	4	3	6	0	0	9	0	0	0	0	0	0	1	2	0	3	16
11:15 AM	0	0	4	0	4	9	4	0	0	13	1	0	0	0	1	0	1	0	0	1	19
					l l															1	
11:30 AM	0	0	3	1	4	5	7	0	0	12	0	1	2	0	3	1	2	0	1	4	23
11:45 AM	1	0	3	0	4	4	4	1_	0	9	0	0	0	0	0	1	4	0	0	5	18_
Total	2	0	13	1	16	21	21	1	0	43	1	1	2	0	4	2	8	2	1	13	76
12:00 PM	0	0	7	0	7	7	7	1	0	15	2	2	2	0	6	0	2	2	0	4	32
12:15 PM		0	4		6	5	4	i	0	10	1	1	2	0	4	2	5	2	0	9	29
	2	_		0					_										-		
12:30 PM	0	0	5	2	7	5	4	1	0	10	0	0	0	2	2	0	3	0	0	3	22
12:45 PM	1	0	10	0	11	3	7	2	2	14	1	0	1_	0	2	2	7	0	1_	10	37
Total	3	0	26	2	31	20	22	5	2	49	4	3	5	2	14	4	17	4	1	26	120
BREAK																					
00.00 DM	4	0	40	0	40.1	4	0	4	0	40	Λ	4	0	0	41	Λ	_	4	0	9	26
02:00 PM	1	0	12	0	13	4	8	1	0	13	0	1	0	0	1	0	5	4	0		36
02:15 PM	2	0	13	0	15	18	3	2	1	24	1	0	0	0	1	0	6	3	0	9	49
02:30 PM	2	1	6	0	9	27	10	5	7	49	1	1	0	0	2	0	15	3	1	19	79
02:45 PM	1	1	4	0	6	10	7	0	2	19	2	0	2	0	4	0	8	3	1	12	41
Total	6	2	35	0	43	59	28	8	10	105	4	2	2	0	8	0	34	13	2	49	205
02.00.014		0	0	0	- I	7		2	0	40		^	4	0	a l	4	10	3	0	14	39
03:00 PM	1	0	6	0	7	7	4	3	<u>2</u> 9	16	. 1	0	1	0	_ <u>2</u> 8	1	10			1	
03:15 PM	7	0	12	0	19	10	15	1		35	6	0	2	0	1	0	8	4	3	15	77
03:30 PM	5	0	6	0	11	9	16	0	2	27	7	6	1	0	14	2	8	3	0	13	65
03:45 PM	3	1	10	0	14	14	7	1	3	25	0	1	1_	. 0	2.	1	7	1_	1	10	51
Total	16	1	34	0	51	40	42	5	16	103	14	7	5	0	26	4	33	11	4	52	232
					•						104				,						
04:00 PM	3	0	11	0	14	15	7	0	_0	_22	3	0	3	0	_6	_ 0	5	3	1	9	51_
04:15 PM	5	0	8	0	13	8	4	1	1	14	2	1	1	0	4	0	6	0	0	6	37
04:30 PM	3	0	6	0	9	6	5	1	2	14	1	1	0	0	2	0	5	4	0	9	34
04:45 PM	3	Ö	12	0	15	11	9	0	0	20	2	0	Ō	ō	2	0	12	1	1	14	51
Total	14	0	37	0	51	40	25	2	3	70	8	2	4	0	14	- 0	28	8	2	38	173
TOTAL	1	U	31	U	51	1 40	20	2	3	, ,	-	_	7	U	171	U	20	Ü	_	00	""
05:00 PM	2	0	12	0	14	13	11	2	0	26	7€ 3	1	2	0	6	0	7	3	0	10	56
05:15 PM	3	0	7	0	10	9	5	1	0	15	1	Ö	0	0	1	0	3	3	0	6	32
		_				-				14		0		0		0	7	1	0	8	34
05:30 PM	1	0	11	0	12	7	7	0	0		0		0	_	0	_			_	-	
05:45 PM	1_	0	9	0	10	6	5	1_	0	12	1	1	0	0	2	0	2	1	0	3	27
Total	7	0	39	0	46	35	28	4	0	67	5	2	2	0	9	0	19	8	0	27	149
Grand											l				I						
Total	61	6	263	4	334	301	204	37	34	576	41	18	22	3	84	15	208	70	27	320	1314
Apprch %	18.3	1.8	78.7	1.2		52.3	35.4	6.4	5.9		48.8	21.4	26.2	3.6		4.7	65.0	21.9	8.4		
Total %	4.6		20.0	0.3	25.4	22.9		2.8	2.6	43.8		1.4	1.7	0.2	6.4		15.8	5.3	2.1	24.4	

Counter:

Counted By:James & Lynn Weather:Clear

Other:

File Name: PARSLE~3

Site Code : 00020509

Start Date : 2/5/2009

Page No : 1

Other:			$-\sqrt{z}$			_				٥.			rag	e No	. !	
			<u>`</u>		1	Groups			s - Axels	2+						
			Parsley rom Nor					Jefferse rom Eas					Parsley om Sou			
Start Time	Right	Thru	Left	Peds	App.	Right	Thru	Left	Peds	App.	Right	Thru	Left	Peds	Арр.	Int.
					Total	_	1	1		Total	-	l			Total	Total
Factor 07:00 AM	1.0	1.0	1.0 19	1.0	29	1.0 35	1.0	1.0 7	1.0	42	1.0	1.0 29	1.0	1.0	31	102
07:00 AM	0	17	10	0	27	25	0	5	0	30	1	19	Ö	0	20	77
07:13 AM	0	19	12	0	31	25	0	ő	0	25	i	21	ő	0	22	78
07:45 AM	0	27	11	0	38	33	0	3	0	36	2	42	ő	0	44	118
Total	0	73	52	0	125	118	0	15	0	133	6	111	0	0	117	375
08:00 AM	0	28	7	0	_35	16	0	11	0	27	5	24	0	1	30	92
08:15 AM	Ö	10	14	Ö	24	27	0	3	0	30	5	26	0	0	31	85
08:30 AM	ő	18	6	Ö	24	18	Ö	2	ō	20	2	17	0	Ō	19	63
08:45 AM	Ö	17	. 11	Ö	28	11	Ō	1	ō	12	1	15	0	0	16	56
Total	0	73	38	0	111	72	0	17	0	89	13	82	0	1	96	296
BREAK																
11:00 AM	0	24	9	0	33	8	0	3	0	11	2	31	0	0	33	77
11:15 AM	0	35	10	0	45	11	0	0	0	11	3	20	ő	0	23	79
11:30 AM	0	20	. 6	0	26	7	0	5	0	12	4	28	ő	0	32	70
11:45 AM	0	18	11	Ö	29	7	0	1	0	8	4	27	ő	ő	31	68
Total	0	97	36	0	133	33	0	<u>'</u>	0	42	13	106	0	0	119	294
40-00 DM	0	20	20	0		44	0	4	0	15	3	14	0	0	17	87
12:00 PM	0	33	22	0 0	55 36	11 19	0 0	4 5	0	24	3	29	0	0	32	92
12:15 PM	0	25	11		39		_		_	23		29	0	0	25	92 87
12:30 PM	0	25	14 9	0	39	18 23	0 0	5	0 0	23 25	4 3	21 26	0	0	29	84
12:45 PM Total	0	21 104	9 56	0	160	23 71	0	2 16	0	87	13	90	0	0	103	350
Total	Ū	104	50	J	100	''	Ū	10	Ŭ	01	10	00	·	Ü	100	000
BREAK																
02:00 PM	0	21	13	0	34	14	0	4	0	18	5	14	0	0	19	71
02:15 PM	0	25	13	0	38	5	0	1	0	6	5	24	0	0	29	73
02:30 PM	0	20	17	1	38	21	0	7	0	28	6	25	0	0	31	97
02:45 PM	0	35	16	0	51	13	0	6	0	19	3	31	0	0	34	104
Total	0	101	59	1	161	53	0	18	0	71	19	94	0	0	113	345
03:00 PM	0	47	24	1	72	2 የ ^ዓ 15	0	3	1	19	4	34	0	0	38	129
03:15 PM	0	47	21	0	_ 68	15	0	7	0	22	7	28	0	0	35	125
03:30 PM	0	22	11	0	33	15	0	4	0	19	12	79	0	0	91	143
03:45 PM	0	18	22	0	40	22	0	5	0	27	9	25	0	0	34 .	101
Total	0	134	78	1	213	67	0	19	1	_87	32	166	0	0	198	498
04:00 PM	0	27	24	0	51		0	3	0	10	6	29	0	0	35	96
04:15 PM	0	35	27	0	62	5	0	4	0	9	4	25	0	0	29	100
04:30 PM	0	28	21	0	49	15	0	3	0	18	6	26	0	0	32	99
04:45 PM	0	24	31	_0_	55	14	0	3 13	0	17	5 21	24 104	0	0	29 125	101 396
Total	0	114	103	0	217	41	0	13	U	54	21	104	U	U		396
05:00 PM	0	19	29	0	48	11	0	2	0	13	2	25	0	0	27	88
05:15 PM	0	28	26	0	54	14	0	2	0	16	4	19	0	0	23	93
05:30 PM	0	19	17	0	36	7	0	3	0	10	3	16	0	0	19	65 54
05:45 PM	0	16_ 82	13 85	0	29 167	5 37	0	3 10	0	<u>8</u> 47	13	13 73	0	0	17 86	<u>54</u> 300
Total	U	82	85	U			U		U					_		
Grand Total	0	778	507	2	1287	492	0	117	1	610	130	826	0	1	957	2854
Approh %	0.0	60.5	39.4	0.2	AE A	80.7	0.0	19.2	0.2	24.4	13.6 4.6	86.3 28.9	0.0		33.5	
Total %	0.0	27.3	17.8	0.1	45.1	17.2	0.0	4.1	0.0	21.4	1 4.0	۷٥.5	0.0	0.0	33.0	

Counter: Counted By:James & Lynn

Weather: Clear Other: Demming & 5th ST Closed

Groups Printed Sundance - Vehicles 2+

File Name: PARSLE~2 Site Code: 00000000 Start Date: 2/19/2009

Page No : 1

Ott ICI. D		mig c	X Out	O I	Ciose	,u	_	·	Duinte			احتمام/	0.	1			ı uç	JC 140		•	
				M						d- Sunda	ance -				-						
			rsley E					iundan rom Ea					rsley E om So		ŀ		Fr	om We	aet .	1	
	Rig	Thr		Ped	App.	Rig	Thr	1	Ped	App.	Rig	Thr		Ped	App.	Rig	Thr		Ped	App.	Int.
Start Time	ht	u	Left	s	Total	ht	u l	Left	s	Total	ht	u	Left	s	Total	ht	u	Left	s	Total	Total
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
07:00 AM	0	10	2	0	12	3	0	9	0	12	13	27	0	0	40	0	0	0	0	0	64
07:15 AM	0	14	1	0	15	2	0	11	0	13	8	35	0	0	43	0	0	0	0	0	71
07:30 AM	0	20	1	0	21	_ 3	0	4	0	7	2	41	0	0	43	0	0	0	0	0	71
07:45 AM	0	20	0	0	20	8	0	4	0	12	4	53	0	0	57	0	0	0	0	0	89
Total	0	64	4	0	68	16	0	28	0	44	27	156	0	0	183	0	0	0	0	0	295
	_			_			_	_	_		_		_			_	_	_			
08:00 AM	0	21	1	0	22	2	0	4	0	6	5	29	0	0	34	0	0	0	0	0	62
08:15 AM	0	17	1	0	18	2	0	5	0	7	0	39	0	0	39	0	0	0	0	0	64
08:30 AM	0	27	0	0	27	1 1	0	4	0	5	2 3	13	0	0 0	15 16	0	0	0	0	0	47 40
08:45 AM Total	0	17 82	<u>1</u> 3	0	18 85	6	0	5 18	0	6 24	10	13 94	0	0	104	0	0	0	0	0	<u>40</u> 213
Total	U	02	3	U	00	0	U	10	U	24	10	34	U	U	104	U	U	U	U	ΟŢ	210
BREAK																					
10:45 AM	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	2
Total	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	2
11:00 AM	1	27	2	0	30	1	0	3	0	4	1	25	0	0	26	0	0	0	0	0	60
11:15 AM	0	22	1	0	23	1	0	3	0	4	1	18	0	0	19	0	0	0	0	0	46
11:30 AM	0	34	2	0	36	1	0	1	0	2	4	20	0	0	24	0	0	0	0	0	62
11:45 AM	0	29	2	0	31	3	0	1	0	4	1_	29	0	0	30	0	0	0	0	0	65
Total	1	112	7	0	120	6	0	8	0	14	7	92	0	0	99	0	0	0	0	0	233
12:00 PM	0	30	2	0	32	1	0	5	0	6	2	28	0	0	30 l	0	0	0	0	0	68
12:15 PM	0	29	0	0	29	Ó	0	6	0	6	5	35	0	0	40	Ö	Ö	Ö	ő	ő	75
12:30 PM	0	30	2	0	32	3	Ö	2	Ö	5	ő	30	0	Ö	30	Ö	Ö	Ö	Ö	ő	67
12:45 PM	Ö	22	2	Ö	24	3	Ō	3	Ö	6	5	30	0	Ö	35	Õ	Õ	Ö	Ō	ō	65
Total	0	111	6	0	117	7	0	16	0	23	12	123	0	0	135	0	0	0	0	0	275
BREAK																					
	_			_			_	_	_	- 1	_		_	_	!	_	_	_	_		
02:00 PM	0	16	0	0	16	0	0	0	0	0	2	20	0	0	22	0	0	0	0	0	38
02:15 PM	0	15	3	0	18	1	0	3	0	4	6	12	0	0	18	0	0	0	0	0	40
02:30 PM	0	22	1	0	23	2	0	10	0	12	0	21	0	0	21 34	0	0	0	0	0	56
02:45 PM Total	0	26 79	- 3 7	0	29 86	5 8	0	<u>5</u> 18	0	10 26	5 13	29 82	0	0	95	0	0	0	0	0	<u>73</u> 207
Total	U	19	′	U	80	1 0	U	10	U	20	13	02	U	U	90	128	U	U	U	0	201
03:00 PM	0	46	2	0	48	1	0	3	0	4	2	31	0	0	33	0	0	0	0	0	85
03:15 PM	Ō	28	2	Ō	30	1	0	4	0	5	3	28	0	0	31	0	0	0	0	0	66
03:30 PM	0	50	1	0	51	1	0	14	0	15	7	23	0	0	30_	. 0	0	0	0	0	96
03:45 PM	0	39	5	0	44	3	0	7	0	10	7	21	0	0	28	0	0	0	0	0	82
Total	0	163	10	0	173	6	0	28	0	34	19	103	0	0	122	0	0	0	0	0	329
04.60 511	_			_			_	_	_	_			_	_	00 1	_	^	_	_	<u>~ </u>	20
04:00 PM	0	30	4	0	34	2	0	5	0	7	2	20	0	0	22	0	0	0	0	0	63
04:15 PM	0	30	7	0	37	1	0	4	0	5	5	16	0	0	21	0	0	0	0	0	63
04:30 PM	0	39	4	0	43	2	0	3	0	5	4	17 15	0	0	21	0	0	0	0	0	69 54
04:45 PM Totai	0	27 126	5 20	0	32 146	5	0	<u>4</u> 16	0	21	14	<u>15</u> 68	0	0	18 82	0	0	0	0	0	54 249
iotai	U	120	20	U	140	1 3	U	10	U	۷۱ ا	14	00	U	U	02	U	U	U	U	U	243
05:00 PM	0	51	7	0	58	2	0	6	0	8	3	17	0	0	20	0	0	0	0	0	86
05:15 PM	Ö	41	3	0	44	. 2	Ö	4	Õ	6	4	18	Õ	Ö	22	Ö	Ö	Ö	Õ	Ö	72
05:30 PM	Ō	31	2	Ō	33	1	0	3	0	4	4	15	0	0	19	0	0	0	0	0	56
05:45 PM	0	24	2	0	26	1	0	2	0	3	1	15	0	0	16	0	0	0	0	0	45
Total	0	147	14	0	161	6	0	15	0	21	12	65	0	0	77	0	0	0	0	0	259
Grand					_	1 -				_	l										
Total	1	885	71	0	957	60	0	147	0	207	114	784	0	0	898	0	0	0	0	0	2062
Apprch %	0.1	92.5	7.4	0.0		29.0	0.0	71.0	0.0		12.7	87.3	0.0	0.0		0.0	0.0	0.0	0.0		
Total %		42.9	3.4	0.0	46.4	2.9	0.0	7.1	0.0	10.0	5.5	38.0	0.0	0.0	43,5	0.0	0.0	0.0	0.0	0.0	

Counter:

Counted By:James & Lynn

Weather: Clear

Other: Demming & 5th ST Closed

File Name: PARSLE~1

Site Code : 00000000 Start Date : 2/19/2009

Page No : 1

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Other, D	CHILL	ing (x 5(1)	01		·u		Grou	ns Prin	ited- Pin	to - Ve	hicles	2+	1	/		ıuç	jo ivo		•	
		Pa	arsley E	Blvd	Ť			Pinto		ited 7 iii	io ve		rsley E		1						
			rom No				F	rom Ea	ast				om So				Fr	om We	est		
Start Time	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr	Left	Ped s	App. Total	Int. Total
Factor	1.0	1.0	1.0	1.0	Total	1.0	1.0	1.0	1.0	rotar	1.0	1.0	1.0	1.0	Total	1.0	1.0	1.0	1.0	Total	- Otal
07:00 AM	1	19	0	0	20	4	0	8	0	12	1	33	2	0	36	0	0	0	0	0	68
07:15 AM	0	25	0	0	25	7	0	4	Ō	11	0	36	0	0	36	1	0	0	0	1	73
07:30 AM	Ö	23	1	Ö	24	4	0	0	Ō	4	0	39	2	0	41	2	0	0	Ō	2	71
07:45 AM	ō	23	1	0	24	1	0	4	Ō	5	6	56	0	0	62	1	0	0	0	1	92
Total	1	90	2	0	93	16	0	16	0	32	7	164	4	0	175	4	0	0	0	4	304
08:00 AM	1	23	2	0	26	10 J	0	3	0	6	1	31	0	0	32	1	0	0	0	1	65
08:15 AM	Ó	20	1	Ō	21	3	0	0	Ō	3	1	36	2	0	39	1	0	0	0	1	64
08:30 AM	0	30	1	0	31	1	0	1	0	2	1	13	1	0	15	1	0	0	0	1	49
08:45 AM	0	22	0	0	22	0	0	1	0	1	0	15	0	0	15	2	0	0	0	2	40
Total	1	95	4	0	100	7	0	5	0	12	3	95	3	0	101	5	0	0	0	5	218
BREAK																					
10:45 AM	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	2
Total	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	0	0	. 0	0	0	2
11:00 AM	0	30	0	0	30	2	0	0	0	2	3	24	1	0	28	2	0	0	0	2	62
11:15 AM	0	21	4	0	25	0	0	2	0	2	3	19	1	0	23	1	0	0	0	1	51
11:30 AM	0	33	2	0	35	1	0	1	0	2	1	23	0	0	24	0	0	1	0	1	62
11:45 AM	0	30	0	0	30	4	0	2	0	6	1	26	1	0	28	1	0	0	0	1	65
Total	0	114	6	0	120	7	0	5	0	12	8	92	3	0	103	4	0	1	0	5	240
12:00 PM	2	35	0	0	37	1	0	0	0	1	1	29	1	0	31	0	0	1	0	1	70
12:15 PM	0	34	1	0	35	1	0	0	0	1	0	39	0	0	39	1	0	1	0	2	77
12:30 PM	0	31	1	0	32	1	0	1	0	2	1	29	0	0	30	1	0	0	0	1	65
12:45 PM	0	25	0	0	25	0	0	0	0	0	1	35	0	0	36	0	0	0	0	0	61
Total	2	125	2	0	129	3	0	1	0	4	3	132	1	0	136	2	0	2	0	4	273
BREAK																					
02:00 PM	0	14	2	0	16	0	0	1	0	1	2	22	0	0	24	2	0	0	0	2	43
02:15 PM	0	17	1	0	18	0	0	4	0	4	2	18	1	0	21	0	0	0	0	0	43
02:30 PM	0	29	3	0	32	0	0	1	0	1	0	21	0	0	21	_ 1	0	0	0	1	55
02:45 PM	0	30	1	0	31	3	0	1	0	4	4	31	0	Õ	35	_ 1	0	0	0	1	71
Total	0	90	7	0	97	3	0	7	0	10	8	92	1	0	101	4	0	0	0	4	212
03:00 PM	0	48	1	0	49	3	0	3	0	6	0	30	0	0	30	0	0	0	0	0	85
03:15 PM	0	30	2	0	32	1	0	4	0	5	7	30	0	0	37	0	0	0	0	0	74
03:30 PM	0	64	2	0	66	3	0	2	0	5	2	27	0	0	29	0	0	0	0	0	100
03:45 PM	0	38	7	0	45	2	0	2	0	4	5	26	1	Ō	32	⁻ 0	0	0	0	0	81
Total	0	180	12	0	192	9	0	11	0	20	14	113	1	0	128	0	0	0	0	0	340
04:00 PM	0	33	2	0	35	1	0	1	0	2	2	21	0	0	23	0	0	2	0	2	62
04:15 PM	0	31	3	0	34	5	0	2	0	7	3	16	0	0	19	0	0	0	0	0	60
04:30 PM	0	37	5	0	42	0	0	0	0	0	2	21	1	0	24	0	0	0	0	. 0	66
04:45 PM	0	30	1	0	31	2	0	0	0	2	2	16	0	0	18	0	0	0	0	0	51
Total	0	131	11	0	142	8	0	3	0	11	9	74	1	0	84	0	0	2	0	2	239
05:00 PM	0	54	3	0	57	2	0	3	0	5	2	18	0	0	20	1	0	0	0	1	83
05:15 PM	2	40	5	Ō	47	1	0	2	Ō	3	6	21	0	0	27	1	0	0	0	1	78
05:30 PM	0	21	3	0	24	1	0	1	0	2	1	18	0	0	19	0	0	0	0	0	45
05:45 PM	0	25	1	0	26	2	0	1	0	3	1	14	0	0	15	1	0	0	0	1	45
Total	2	140	12	0	154	6	0	7	0	13	10	71	0	0	81	3	0	0	0	3	251
Grand	6	966	56	0	1028	59	0	55	0	114	62	834	14	0	910	22	0	5	0	27	2079
Total	_			_	1028					114					910		_			21	2019
Apprch %		94.0 46.5	5.4 2.7	0.0	49.4	51.8 2.8	0.0	48.2 2.6	0.0	5.5	6.8 3.0	91.6 40.1	1.5 0.7	0.0	43.8	81.5 1.1	0.0	18.5 0.2	0.0	1.3	
Total %	0.3	40.5	2.1	U.U	49.4	2.8	0.0	2.0	0.0	5.5	3.0	4U. I	0.7	0.0	43.0	1.1	0.0	0.2	0.0	1.3	i

Parsley South of I 80

Data For Site: Parsley South Storage Mode: Count Data collected by a Apollo (ver 1.45) Posted speed: 40 Record Interval Length: 15 min

	NB	SB	ı	NB	SB		NB	SB		NB :	SB
0:00	7	8	6:00	31	24	12:00	25	16	18:00	24	56
15	6	7	15	33	20	15	28	17	15	21	45
30	6	5	30	29	21	30	26	17	30	12	31
45	5	5	45	35	26	45	29	15	45	19	26
1:00	5	8	7:00	38	30	13:00	22	20	19:00	15	25
15	5	7	15	67	65	15	29	23	15	18	19
30	6	5	30	75	61	30	24	25	30	21	15
45	7	6	45	76	63	45	26	35	45	11	14
2:00	7	. 4	8:00	70	65	14:00	31	27	20:00	4	13
15	5	5	15	59	55	15	22	34	15	2	11
30	3	4	30	52	40	30	20	48	30	5	6
45	6	3	45	51	31	45	26	37	45	3	5
3:00	4	2	9:00	49	35	15:00	21	37	21:00	2	3
15	2	3	15	36	29	15	18	48	15	1	4
30	2	2	30	30	28	30	19	39	30	2	2
45	4	1	45	28	23	45	21	35	45	1	3
4:00	3	1	10:00	29	25	16:00	17	26	22:00	1	2
15	5	5	15	26	23	15	29	32	15	1	1
30	6	6	30	28	25	30	28	34	30	1	1
45	8	8	45	26	24	45	17	30	45	2	1
5:00	9	7	11:00	25	26	17:00	16	40	23:00	5	
15	10	9	15	23	28	15	20	49	15	3	1
30	11	8	30	25	23	30	19	56	30	2	1
45	15	14	45	24	22	45	24	72	45	1	2

Data For Site:

Parsley Nirth-of Sundance

Storage Mode : Count

Data collected by a Apollo (ver 1.45) Posted speed : 40 Record Interval Length: 15 min

	NB	SB	I	NB	SB		NB	sb \		NB	CD.
0:00	6	9	6:00	28	31	12:00	24	21	18:00	4	SB 10
15	5	6	15	31	23	15	26	17	15	2	3
30	5	7	30	28	24	30	26	16	30	2	5
45	4	7	45	$\frac{-25}{31}$	28	45	26	15	45	$\frac{2}{9}$	$\frac{-\frac{3}{6}}{6}$
1:00	3	5	7:00	35	39	13:00	22	20	19:00	5	
15	4	5	15	66	40	15	29	21	15.00	8	$\frac{3}{9}$
30	5	7	30	74	55	30	23	22	30	11	$\frac{9}{7}$
45	6	5	45	78	57	45	26	27	45	10	 /8
2:00	5	5	8:00	70	46	14:00	34	31	20:00	4	1
15	2		15	56	49	15	28	36	15	3	2
30	1	1	30	51	44	30	17	52	30	4	2
45	5	2	45	40	35	45	22	39	45	2	$\frac{2}{2}$
3:00	3	5	9:00	39	31	15:00	21	30	21:00	<u>2</u> 1	$\frac{2}{1}$
15	2	1	15	32	40	15	17	41	15	2	2
30	2	ī	30	28	25	30	21	35	30	1	$\frac{2}{1}$
45	3	2	45	25	26	45	19	31	45	2	2
4:00	4	2	10:00	27	29	16:00	19	22	22:00	1	$\frac{2}{1}$
15	6	4	15	24	25	15	30	24	15	2	2
30	5	5	30	26	31	30	21	28	30	2	$\frac{2}{2}$
45	2	9	45	25	29	45	19	31	45	1	1
5:00	6	. 10	11:00	28	29	17:00	19	33	23:00	2	2
15	8	11	. 15	26	26	15	22	47	15	2	
30	10	11	30	21	32	30	26	46	30	3	1
45	14	12	45	23	34	45	24	55	45	2	2
		-							1		<u> </u>

Data For Site : Pinto East of Parsley

Storage Mode : Count
Data collected by a Apollo (ver 1.45)
Posted speed : 30 Record Interval Length: 15 min

	EB	WB		ΕB	WB		EB	WB		EΒ	WB
0:00	1	1	6:00	1	2	12:00	5	10	18:00		2
15			15	2	6	15	4	11	15	3	1
30			30	2	8	30	3	3	30	2	1
45		_	45	3	7	45	2	9	45	1	1
1:00	1		7:00	4	12	13:00	2	10	19;00	2	
15			15	5	29	15	5	16	15	2	$\frac{2}{3}$
30		-	30	11	26	30	5	11	30	1	$\frac{}{3}$
45	1	1	45	9	27	45	7	9	45	2	3
2:00			8:00	7	29	14:00	7	17	20:00	2	1
15	1	1	15	8	15	15	7	15	15	1	1
30			30	6	15	30	19	15	30	2	1
45		1	45	5	18	45	15	9	45	1	2
3:00	1		9:00	5	10	15:00	26	8	21:00	1	1
15			15	4	14	15	15	9	15	1	1
30	_		30	3	17	30	15	7	30		
45		-	45	3	5	45	14	5	45	1	
4:00			10:00	1	4	16:00	19	8	22:00	· <u>-</u>	
15			15	1	3	15	38	9	15		 1
30	1	1	30	1	4	30	27	7	30	1	
45		1	45	2	2	45	26	6	45		1
5:00		1	11:00	1	3	17:00	17	11	23:00	1	
15	2	1	15	3	2	15	24	12	15	1	1
30	2	2	30	2	2	30	10	9	30	2	
45	1	3	45	2	3	45	12	7	45	1	1
						_			1		

Dată For Site: Allison between Walterschied & South Greeely

Storage Mode : Count
Data collected by a Apollo (ver 1.45)
Posted speed : 30
Record Interval Length : 15 min

	EB	WB I	[EB	WB		EB	WB		EB	WB
0:00	1		6:00	5	2	12:00	12	15	18:00	5	7
15			15	12	7 .	15	16	16	15	1	6
30	2	1	30	15	5	30	14	18	30	3	4
45		1	45	20	6	45	18	14	45	5	5
1:00			7:00	29	12	13:00	12	19	19:00	7	10
15			15	51	19	15	17	22	15	10	5
30	1		30	59	33	30	14	23	30	11	6
45			45	54	39	45	18	31	45	9	7
2:00	3	2	8:00	47	26	14:00	19	29	20:00	5	2
15			15	56	31	15	21	34	15	4	4
30	1		30	48	32	30	19	49	30	6	6
45		1	45	38	21	45	20	38	45	5	2
3:00			9:00	31	16	15:00	22	36	21:00	2	1
15			15	21	14	15	26	47	15	1	1
- 30	2		30	22	13	30	21	39	30	2	1
45	1	1	45	9	7	45	19	35	45	1	
4:00			10:00	18	5	16:00	17	24	22:00		1
15		1	15	7	4	15	24	26	15	2_	1
30			; 30	9	6	30	21	49	30	1	2
45	2		45	5	4	45	18	29	45		1
5:00	10		11:00	9	9	17:00	19	32	₹23:00	1	1
15		3	15	3	5	15	22	48	15	1_	
30	6	5	30	2	11	30	26	44	30	2	1
45	4	2	45	10	11	45_	21	51	45		1
			-			;			No de Care		

Data For Site:

Parsley North of Jazz Dr

Storage Mode : Count
Data collected by a Apollo (ver 1.45) Posted speed: 40
Record Interval Length: 15 min

	NB	SB		NB	SB		NB	SB		NB	SB
0:00	5	7	6:00	29	29	12:00	23	15	18:00	5	7
15	4	5	15	32	24	15	25	16	15	1	6
30	5	5	. 30	27	31	30	25	18	30	3	4
45	2	6	45	33	26	45	27	14	45	6	4
1:00	4	5	7:00	36	33	13:00	21	19	19:00	6	9
15	3	4	15	65	36	15	29	22	15	7	5
30	4	6	30	71	59	30	22	23	30	9	6
45	4	5	45	74	58	45	25	31	45	8	8
2:00	3	5	8:00	69	43	14:00	29	29_	20:00	2	2 2
15	2	4	15	54	42	15	21	34	15	3	
30	2	3	30	49	42	30	18	49	30	5	3
45	4	5	45	45	36	45	23	38	45	3	2
3:00	2	4	9:00	44	27	15:00	20	36	21:00	1	1
15	1	2	15	33	38	15	16	47	15	1	1
30	1	2	30	29	26	30	22	39	30	1	1
45	2	1	45	24	25	45	18	35	45	1	
4:00	2	2	10:00	28	27	16:00	16	24	22:00	1	1
15	2	2	15	23	23	15	26	26	15	1	1
30	2	2	30	27	29	30	17	49	30	1	2
45	1	1	45	24	25	45	18	29	45	1	1
5:00	3	5	11:00	29	28	17:00	17	32	23:00	2	1
15	6	6	15	22	25	15	21	48	15	1	
30	9	10	30	22	31_	30	24	44	30	2	1
45	12	8	45	21	33	45	18	51	45	11	1

Data For Site: Cribbon Between Woodward & Gopp Ct

Storage Mode : Count

Data collected by a Apollo (ver 1.45)
Posted speed: 30 Record Interval Length: 15 min

	NB	SB	•	NB	SB]	NB	SB		NB	SB
0:00	1	1	6:00	2	3	12:00	10	12	18:00	12	1
15	2	1	15	3	5	15	9	10	15	2	1
30	1	1	30	4	4	30	8	7	30	1	2
45	2	1	45	6	10	45	8	9	45	3	3
1:00	1	1	7:00	5	17	13:00	10	6	19:00	5	2
15			15	8	22	15	9	9	15	4	3
30			30	15	29	30	13	7	30	1	5
45		1	45	16	26	45	16	7	45	3	4
2:00	2	1	8:00	13	25	14:00	19	11 🖁	20:00	4	1
15	1	1	15	15	29	15	22	10	15	2	2
30	1		30	19	22	30	25	9	30	1	2 3 5 2
45	•		45	10	16	45	26	11	45	2	5
3:00	1	1	9:00	12	18	15:00	46	14	21:00	1	2
15	2	1	15	9	12	15	42	12	15	2	1
30			30	7	10	30	30	9	30	1	
45			45	6	11	45	26	10	45		1
4:00	2	1	10:00	4	6	16:00	15	16	22:00	1	1
15	1	1	15	6	5	15	14	12	15	1	2
30	2	1	30	5	2	30	24	10	30		
45			45	2	5	45	19	10	45	1	1
5:00	11	1	11:00	4	6	17:00	18	8	23:00	1	
15	2	2	15	2	4	15	16	7	15	1	
30	4	3	30	3	5	30	17	9	30	1	2
45	6	5	45	5	6	45	19	10	45	1	1
								and the same of th			

Data For Site: Allison Between Walterschied and Desmet Storage Mode: Count
Data collected by a Apollo (ver 1.45)
Posted speed: 30
Record Interval Length: 15 min

_	EB	WB	1	EB	WB	I	EB	WB 5		EB	WB
0:00	1	2	6:00	29	19	12:00	12	15 🖁	18:00	19	37
15	2	_1	15	22	14	15	16	16	15	19	35
30	2	1	30	17	21	30	14	18	30	18	36
45	2	1	45	23	16	45	18	14	45	14	29
1:00	1	2	7:00	26	23	13:00	12	19	19:00	8	26
15	3	1	15	55	26	15	17	22	15	9	15
30	1	4	30	61	49	30	14	23	30	11	16
45	1	2	45	64	48	45	18	31	45	9	12
2:00	1	1	8:00	63	33	14:00	19	29	20:00	7	10
15	1	1	15	54	32	15	21	34	15	6	6
30	1	1	30	45	31	30	19	49	30	3	7
45		1	45	41	26	45	20	38	45	8	5
3:00	1	2	9:00	45	17	15:00	22	36	21:00	2	3
15	1	1	15	31	28	15	26	47	15	2	2
30	1	2	30	26	16	30	21	39	30	2	1
45	1	1	45	18	15	45	19	35	45	1	2
4:00	2	1	10:00	14	17	16:00	17	24	22:00	2	2
15	1	1	15	15	13	15	24	26	15	4	2
30	2	1	30	18	19	30	21	49	30	. 1	1
45	2		45	15	15	45	18	29	45	1	1
5:00	5	5	11:00	19	18	17:00	19	32	23:00	2	1
15	4	8	15	12	1.5	15	22	48	15		2
30	12	11	30	11	21	30	26	44	30	3	
45	9	8	45	11	23	45	21	51	45		2

Allison Between Aherns & Cribbon

Data For Site: Allison Betwee Storage Mode: Count Data collected by a Apollo (ver 1.45) Posted speed: 30 Record Interval Length: 15 min

	EB	WB		EB	WB		EB	WB		EB	WB
0:00		1	6:00	1	2	12:00	7	7	18:00	4	3
15	-		15	4	6	15	8	9	15	3	2
30	1	1	30	3	7	30	9	8	30	3	1
45	1	1	45	2	9	45	7	8	45	2	3
1:00			7:00	7	15	13:00	9	5	19:00	6	4
15		1	15	9	25	15	11	8	15	3	4
30	1		30	17	28	30	12	6	30	2	
45	1		45	18	26	45	15	6	45	4	5
2:00	1	1	8:00	13	24	14:00	15	9	20:00	2	$ \begin{array}{r} $
15	_		15	16	27	15	15	11	15	1	3
30	1	1	30	15	25	30	25	8	30	2	2
45	1		45	11	15	45	20	10	45	3	4
3:00		1	9:00	9	16	15:00	14	12	21:00	2	1
15			15	8	11	15	24	13	15	2	
30	1	1	30	. 6	10	30	19	11	30	1	1
45	1	_ 1	45	5	9	45	18	9	45	1	1
<u>4</u> :00	1		10:00	2	. 8	16:00	13	14	22:00	1	2
15		1	15	1	4	15	15	13	15	2	1
30	1		30	4	5	30	25	11	30	1	1
45		1	45	3	2	45	16	9	45		1
5:00	. 1		11:00	5	4	17:00	19	9	23:00	_ 1	2
15	44	1	15	3	2	15	21	8	15	2	1
30	2	3	30	1	1	30	22	11	30	2	1
45	1	3	45	6	5	45	23	-11	45	1	1
	19	18		169	286		382	226		51	53

0

24 Hour Total

1204

Data For Site:

School cut around

Storage Mode : Count

Data collected by a Apollo (ver 1.45)
Posted speed: Unknown
Record Interval Length: 15 min

	EB	WB		EB	WB		EB	WB		EB	WB
0:00			6:00	1		12:00	2		18:00	2	
15			15			15	1		15		1
30	1	-	30	2		30			30		
45		1	45	2	5	45			45		
1:00	1		7:00	1	12	13:00			19:00		1
15		· · · · · · · · · · · · · · · · · · ·	15	2	15	15	3		15	2	
30		1	30	6	16	30			30		
45			45	15	19	45			45	1	
2:00			8:00	19	21	14:00			20:00		1
15	1	1	15	10	19	15	9	19	15		
30			30	9	18	30	8	16	30	1	
45	1		45	6	7	45	6	15	45	1	
3:00		1	9:00	2	5	15:00	5	12	21:00		1
15			15	1		15		13	15	1	
30			30		2	30	2	2	30		1
45	1	1	45		1	45		1	45	1	_1
4:00			10:00	1		16:00			22:00		
15			15	1	1	15	1	2	15		
30	1		30	1		30	1		30		1
45			45			45	3_	1	45	1	
5:00			11:00		1	17:00	4		23:00		1
15	2	1	15	2		15	3	1	15		
30			30		1	30	1	1	30		
45	2	1	45	2		45	1		45		

Data For Site :

Jefferson between Arp & Parsley

Storage Mode : Count
Data collected by a Apollo (ver 1.45)
Posted speed : 30 Record Interval Length: 15 min

	EB	WB }		EB	WB .	1	EB	WB	i	EB	WB
0:00	1	1	6:00	2	4	12:00	10	10	18:00	6	4
15	1		15	4	12	15	9	11	15	5	1
30		1	30	5	16	30	7	3	30	4	2
45	1	1	45	6	15	45	4	9	45	1	3
1:00	1		7:00	9	25	13:00	6	10	19:00	5	6
15	2	1	15	10	40	15	12	16	15	4	5
30			30	25	38	30	11	11	30	5	3
45	1	,	45	26	36	45	16	9	45	5	2
2:00	2		8:00	15	34	14:00	15	17	20:00	3	4
15	1	1	15	19	31	15	16	15	15	2	1
30		2	30	18	18	30	30	19	30	1	2
45	1	1	45	16	19	45	26	30	45	3	3
3:00	1	1 ,	9:00	12	21	15:00	28	46	21:00	1	2
15	1		15	10	16	15	31	26	15	2	
30			30	8	15	30	29	25	30	2	1
45			45	4	12	45	27	24	45	11	
4:00		1	10:00	3	9	16:00	25	19	22:00	2	2
15	1		15	2	6	15	26	48	15	3	1
30	2	1	30	3	9	30	24	37	30	1	
45	1	2	45	4	5	45_	21	26	45		
5:00	2	1	11:00	3	6	17:00	33	27	23:00		2
15	6	2	15	6	4	15	48	24	15	1	
30	5	4	30	5	5	30	49	10	30	2	
45	3	6_	45	4	6	45	39	12	45	11	

Data For Site:

Allison Between Snyder & Cribbon

Storage Mode : Count
Data collected by a Apollo (ver 1.45)
Posted speed : 30
Record Interval Length : 15 min

	EB	WB (EB	WB /		EB	WB	£	EB	WB
0:00	1	1	6:00	2	3	12:00	13	15	18:00	12	3
15	1		15	11	8	15	15	17	15	5	2
30	2	1	30	15	6	30	17	16	30	2	1
45		1	45	18	4	45	13	15	45	5	6
1:00	1	1	7:00	30	14	13:00	17	11	19:00	10	8
15	1	1	15	53	9	15	20	16	15	7	6
30		2	30	56	34	30	23	14	30	5	4
45		5	45	51	36	45	29	13	45	7	5
2:00		1	8:00	48	26	14:00	28	18	20:00	5	2
15	2	1	15	52	32	15	37	23	15	3	2
30	1	1	30	50	30	30	50	17	30	3	1
45	2	1	45	32	22	45	40	19	45	5	2
3:00			9:00	31	18	15:00	26	25	21:00	3	
15	1		15	23	17	15	32	25	15	3	1
30		2	30	11	16	30	40	21	30	2	2
45		1	45	18	15	45	34	17	45	2	1
4:00	1	1	10:00	15	12	16:00	25	27	22:00	1	2
15	1	1	15	7	10	15	31	24	15	3	1
30	1	1	30	9	8	30	47	23	30	2	1
45		1	45	5	7	45	33	19	45	1	2
5:00	1	2	11:00	8	6	17:00	39	17	23:00	1	2
15	3	3	15	6	7	15	40	16	15	1	2
30	4	4	30	3	4	30	38	22	30	1	2
45	2	4	45	11	13	45	45	20	45	1	1

CRASH HISTORY FOR CHEYENNE AT THE INTERSECTION OF ALLISON RD & CRIBBON AVE

3H 2008	
THE YEARS 2005 THROUGH 2008	
THE YEARS ?	
FOR	

DRIVER ACTION	Failed to Keep Proper Lane Wrong Side/Wrong Way Ran Off Road
ROAD	λά
LIGHT COND	Daylight
MOST HARMFUL EVENT	Utility PolefLight Support
ACTIVITY PRIOR	Straight Ahead
DIRECTION	West
MANNER_OF COLLISION	Not a Collision w/2 Vehicles in Transport
NUM NUM JUNCTION INJ KIL RELATION	1 0 Intersection
CRASH LOCATION	ALLISON RD CRIBBON AVE
REPORT	17922
TIME	613
DATE	03/03/2008

TOTAL CRASHES IN THIS REPORT 1 TOTAL PERSONS INJURED TOTAL PERSONS KILLED PDO CRASHES INJURY CRASHES FATAL CRASHES

CREATED BY: Ann Smith; WYDOT Highway Safety; (307) 777-4258; Ann.Smith@dot.state.wy.us

2/4/2009

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DRIVER ACTION	
ROAD	
COND	
MOST HARMFUL EVENT	
ACTIVITY PRIOR	
DIRECTION	
MANNER_OF COLLISION	TOTAL CRASHES
JUNCTION RELATION	FATAL
NUM NUM JUN INJ KIL REL	INJURY CRASHES
	PDO* CRASHES
CRASH LOCATION	NUMBER PERSONS KILLED
REPORT	NUMBER PERSONS INJURED
TIME	
DATE	

2008 TOTAL

*PDO = Property Damage Only Crashes; No Injuries, No Fatalities

2/4/2009

CRASH HISTORY FOR CHEYENNE AT THE INTERSECTION OF ALLISON RD & DESMET DR FOR THE YEARS 2005 THROUGH 2008

					Close Viving
	DRIVER ACTION		Unknown		Following too Close No Improper Driving
	ROAD		Dry		Ç.
	COND		Darkness Lighted		Daylight
0	MOST HARMFUL EVENT				
OR THE TEARS 2003 THROUGH 2000	ACTIVITY PRIOR		Straight Ahead		Straight Ahead Stopped in Traffic
E IEARO Z	DIRECTION		South		East
בו אטר	MANNER OF COLLISION		Unknown		Rear End (Front to Rear)
	NUM NUM JUNCTION INJ KIL RELATION		0 0 Intersection Related		0 0 Intersection Related
			띪		R
	CRASH		Ö		ALLISON DR DESMET
	REPORT		20402		03793
	TIME		8		729
	DATE	2006	12/09/2006	2007	03/01/2007

TOTAL CRASHES IN THIS REPORT	THIS REPORT	7	
PDO CRASHES	2		
INJURY CRASHES	0		
FATAL CRASHES	0		
TOTAL PERSONS INJURED	NS INJURED	0	
TOTAL PERSONS KILLED	NS KILLED	0	

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DRIVER	
ROAD	
LIGHT	
MOST HARMFUL EVENT	
ACTIVITY PRIOR	
DIRECTION	
MANNER_OF COLLISION	
NUM NUM JUNGTION INJ KIL RELATION	
TIME REPORT CRASH NUMBER LOCATION	
REPORT NUMBER	
TIME	
DATE	

NUMBER PERSONS INJURED	0	0	
NUMBER PERSONS KILLED	0	0	
PDO⁴ CRASHES	1	1	,
INJURY CRASHES	0	0	
FATAL CRASHES	0	0	•
TOTAL	ı	,	

^{*}PDO = Property Damage Only Grashes; No Injuries, No Fatalities

CRASH HISTORY FOR CHEYENNE AT THE INTERSECTION OF ALLISON RD & S GREELEY HWY FOR THE YEARS 2005 THROUGH 2008

						70X 1HL 701	-AKS 200	FOR THE YEARS 2003 THROUGH 2008				
DATE	TIME	REPORT NUMBER	CRASH LOCATION	NUM NUM INJ KIL	JM JUNCTION IL RELATION	MANNER_OF COLLISION	DIRECTION	ACTIVITY PRIOR	MOST HARMFUL EVENT	LIGHT	ROAD	DRIVER ACTION
2005												
04/25/2005	1617	06470	ALLISON DR	0	Intersection Related	Head On (Front to Front)	East	Stopped in Traffic		Daylight	ρ	No Improper Driving
			S.GREELEY				West	Backing				Improper Backing
06/16/2005	2020	09393	ALLISON DR	0) Intersection	Other	East	Straight Ahead		Daylight	Dry	No Improper Driving
			S.GREELEY				South	Straight Ahead				Disregarded Traffic Signs
2006												
03/10/2006	1855	04445	S.GREELEY	2	0 Intersection	Angle Direction not Specified	South	Straight Ahead		Darkness Lighted	Ice/Frost	Drove too Fast for Conditions
			ALLISON DR				North	Straight Ahead				No Improper Driving
04/14/2006	1535	06578	ALLISON DR	0	1 Intersection	Other	North	Turning Left		Daylight	ρλ	Failed to Yield ROW
			S.GREELEY				South	Straight Ahead				No Improper Driving
09/11/2006	720	14506	S.GREELEY	0	Intersection Related	Rear End (Front to Rear)	South	Straight Ahead		Daylight	ογ	Following too Close
			ALLISON DR				South	Slowing				No Improper Driving
10/22/2006	1405	18067	ALLISON DR	1	1 Intersection Related	Angle Direction not Specified	Southeast	Straight Ahead		Daylight	ργ	No Improper Driving
			S,GREELEY				East	Stopped in Traffic				Unknown
12/19/2006	1220	21400	ALLISON DR	0	0 Intersection	Angle Direction not Specified	West	Legacy - Starting in traffic		Daylight	Wet	Unknown
			S.GREELEY				South	Straight Ahead				Disregarded Traffic Signs
12/26/2006	1000	22168	S,GREELEY	0	0 Intersection Related	Rear End (Front to Rear)	South	Stopped in Traffic		Daylight	ģ	No Impraper Driving
			ALLISON DR				South	Straight Ahead				Unknown
2007												
07/01/2007	1830	11091	S.GREELEY	-	0 Intersection Related	Rear End (Front to Rear)	North	Straight Ahead		Daylight	Dry	Other Improper Action
			ALLISON DR				North	Slowing				Other Improper Action
07/28/2007	1648	12908	S.GREELEY	0	0 Intersection Related	Rear End (Front to Rear)	South	Stopped in Traffic		Daylight	ģ	No Improper Driving
			ALLISON DR				South	Slowing				Unknown
09/14/2007	1232	15833	ALLISON DR	0	0 Intersection	Other	South	Straight Ahead		Daylight	Dry	Disregarded Traffic Signs
			S.GREELEY				East	Turning Left				Unknown
2008												
06/03/2008	955	08196	S GREELEY HWY	0	0 Intersection	Angle Right (Front to Side, includes Broads	North	Straight Ahead	Motor Vehicle in Transport on OTHER Road	Daylight	ρ'n	Ran Red Light
			ALLISON RD				West	Turning Right	Motor Vehicle in Transport on OTHER Road			No Improper Driving
08/10/2008	1405	17700	S GREELEY HWY	0	0 Intersection	Angle Same Direction (Front to Side)	South	Straight Ahead	Motor Vehicle in Transport on Roadway	Daylight	Wet	No Improper Driving
			ALLISON RD				South	Stopped in Traffic	Motor Vehicle in Transport on Roadway			Failed to Keep Proper Lane
												Speeding
												Improper Turn or No Signal
												Erratic/Reckless/Careless/Aggre

2/4/2009

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MANNER_OF COLLISION				
JUNCTION RELATION				•
NUM NUM				
UN N				
CRASH		5		4 °
REPORT		THIS REPORT	5 ° 0	NS INJURED NS KILLED
TIME		TOTAL CRASHES IN THIS REPORT	PDO CRASHES INJURY CRASHES FATAL CRASHES	TOTAL PERSONS INJURED TOTAL PERSONS KILLED
DATE		TOT	PD0 INJU	
	1			

DRIVER

ROAD

LIGHT

MOST HARMFUL EVENT

ACTIVITY PRIOR

DIRECTION

	NUMBER PERSONS INJURED	NUMBER PERSONS KILLED	PDO* CRASHES	INJURY	FATAL	TOTAL
2005	0	0	7	5	5	
2006	ო	0	4	7	0	
2007	-	0	7	-	0	
2008	0	0	2	0	0	
TOTAL	4		9	m	0	

*PDO = Property Damage Only Crashes; No Injuries, No Fatalities

2/4/2009

CRASH HISTORY FOR CHEYENNE AT THE INTERSECTION OF ALLISON RD & WALTERSCHEID BLVD FOR THF YFARS 2005 THROUGH 2008

							FOR THE T	EARS ZUI	HE TEAKS 2003 INKOUGH 2000				
DATE	TIME	REPORT	CRASH LOCATION	NUM INJ	NUM JE KIL RI	NUM NUM JUNCTION INJ KIL RELATION	MANNER_OF COLLISION	DIRECTION	ACTIVITY PRIOR	MOST HARMFUL EVENT	LIGHT	ROAD	DRIVER ACTION
2006													
02/17/2006	1703	03327	ALLISON DR WALTERSCHD	-	년 0	0 Intersection Related	Rear End (Front to Rear)	East East	Stopped in Traffic Slowing		Daylight	Snow	No Improper Driving Drove too Fast for Conditions
2007													
01/26/2007	1042	01880	WALTERSCHD	7	<u>e</u>	Intersection	Angle Direction not Specified	East	Straight Ahead		Daylight	Dry	Disregarded Traffic Signs
			ALLISON DR					North	Straight Ahead				No Improper Driving
03/13/2007	1620	04618	WALTERSCHD	4	0	Intersection	Other	West	Turning Left		Daylight	Dry	No Improper Driving
			ALLISON DR					East	Straight Ahead				Unknown
05/24/2007	727	08650	WALTERSCHD	м	0	Intersection	Angle Direction not Specified	East	Straight Ahead		Daylight	Dry	Failed to Yield ROW
			ALLISON DR					North	Straight Ahead				No Improper Driving
								Northwest	Legacy - Starting in traffic				No Improper Driving
11/20/2007	2200	20281	ALLISON DR	-	<u>=</u>	Intersection Related	Unknown	East	Straight Ahead		Darkness Lighted	Snow	Unknown
			WALTERSCHD										
2008													
08/22/2008	1700	18806	WALTERSCHEID BI	0	<u>e</u>	Intersection	Angle (Front to Side), Opposing Direction	East South	Straight Ahead Straight Ahead	Motor Vehicle in Transport on Roadway Motor Vehicle in Transport on Roadway	Daylight	Dıy	Failed to Yield ROW No Improper Driving

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MANNER_OF COLLISION				
NUM NUM JUNCTION INJ KIL RELATION			00'	
NUM				
NUM NUM INJ KIL				
CRASH LOCATION		φ		£ o
REPORT		TOTAL CRASHES IN THIS REPORT	٠ 0	TOTAL PERSONS INJURED TOTAL PERSONS KILLED
TIME		HES IN	SE SE	PERSO
DATE		TOTAL CRAS	PDO CRASHES INJURY CRASHES FATAL CRASHES	TOTAL TOTAL

DRIVER ACTION

ROAD

LIGHT

MOST HARMFUL EVENT

ACTIVITY PRIOR

DIRECTION

NUMBER NUMBER PDO NUJURY FATAL 1 PERSONS PERSONS CRASHES CRASHES CRASHES CRASHES CRASHES CRASHES CASHES CAS	2008	18101
PDO* INJURY FATAL CRASHES CRASHES CRASHES	0	11
INJURY FATAL CRASHES CRASHES 1 0 4 0	0	c
FATAL CRASHES 0	1	-
	0	v
ר מ	0	o
TOTAL CRASHES 1	1	9

*PDO = Property Damage Only Crashes; No Injuries, No Fatalities

2/4/2009

CRASH HISTORY FOR CHEYENNE AT THE INTERSECTION OF PARSLEY BLVD & PINTO LN

FOR THE YEARS 2005 THROUGH 2008

ACTIVITY PRIOR

DIRECTION

DRIVER

ROAD

COND

MOST HARMFUL EVENT

TIME REPORT CRASH NUM NUM JUNCTION MANNER, OF NUMBER LOCATION INJ KIL RELATION COLLISION

DATE

TOTAL CRASHES IN THIS REPORT 0
PDO CRASHES 0
INURY CRASHES 0
FATAL CRASHES 0
TOTAL PERSONS INURED
TOTAL PERSONS KILLED

NUMBER NUMBER PDO- INJURY FATAL TOTAL PERSONS PERSONS CRASHES CRASHES

*PDO = Property Damage Only Crashes; No Injuries, No Fatalities

2/4/2009

CRASH HISTORY FOR CHEYENNE AT THE INTERSECTION OF PARSLEY BLVD & W JEFFERSON RD FOR THE YEARS 2005 THROUGH 2008

DATE	TIME	REPORT	CRASH LOCATION	NUM NU INJ KR	NUM NUM JUNCTION INJ KIL RELATION	MANNER_OF COLLISION	DIRECTION	ACTIVITY PRIOR	MOST HARMFUL EVENT	LIGHT	ROAD	DRIVER ACTION
2005												
07/12/2005	1623	10529	JEFFERSON	0	0 0 Intersection Related	Other	South	Turning Left		Daylight	ογ	Unknown
			PARSLEY BL				South	Stopped in Traffic				No Improper Driving
2002												
03/22/2007	1356	05198	PARSLEY BL	0	Intersection	Other	West	Turning Left		Daylight	Dry	No Improper Driving
			JEFFERSON				South	Turning Left				Improper Turn or No Signal
10/12/2007	1442	17309	PARSLEY BL	0	0 Intersection Related	Rear End (Front to Rear)	North	Straight Ahead		Daylight	<u>6</u>	Following too Clase
			JEFFERSON				North	Stopped in Traffic				No Improper Driving

۰ ۰ TOTAL CRASHES IN THIS REPORT 3 TOTAL PERSONS INJURED TOTAL PERSONS KILLED PDO CRASHES INJURY CRASHES FATAL CRASHES

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LIGHT			
MOST HARMFUL EVENT			
ACTIVITY PRIOR			
DIRECTION			
MANNER_OF COLLISION	TOTAL CRASHES	- 0	6
	FATAL TOTAL CRASHES CRASHES	0 1	8 0
UNCTION		0 0 0	£ 0 0
	INJURY FATAL CRASHES CRASHES	2 0 0 1	3 0 0 3
UNCTION	FATAL CRASHES	0 1 0 0 1	0 0 3
REPORT CRASH NUM NUM JUNCTION NUMBER LOCATION INJ KIL RELATION	NUMBER PERSONS PDO' INJURY FATAL KILED CRASHES CRASHES	0 0 1 0 0 1	0 0 3
CRASH NUM NUM JUNCTION LOCATION INJ KIL RELATION	PDO" INJURY FATAL CRASHES CRASHES	0 0 1 0 0 1	TOTAL 0 0 3 0 0 3

DRIVER

ROAD

*PDO = Property Damage Only Crashes; No Injuries, No Fatalities

Analysis Period (min)

Critical Lane Group

Year 2014

Timing Plan: AM PEAK

10/15/2009 **EBL** Movement **EBT EBR** WBT **WBL WBR NBL NBT** NBR SBL SBT SBR Lane Configurations t, 4 ħ 1900 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Total Lost time (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Frt 1.00 0.94 0.99 1.00 0.98 1.00 1.00 0.85 Fit Protected 0.95 1.00 0.95 1.00 1.00 0.95 1.00 1.00 Satd. Flow (prot) 1770 1754 1848 1770 1826 1770 1863 1583 Flt Permitted 0.68 1.00 0.98 0.72 1.00 0.65 1.00 1.00 Satd. Flow (perm) 1266 1754 1340 1817 1826 1212 1863 1583 Volume (vph) 123 138 162 88 11 244 12 131 20 11 52 289 Peak-hour factor, PHF 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 Adi. Flow (vph) 137 153 98 12 271 13 180 146 22 12 58 321 RTOR Reduction (vph) 9 0 38 0 0 0 0 0 0 0 205 2 Lane Group Flow (vph) 137 213 0 0 294 0 180 159 0 12 58 116 Perm Perm Perm Turn Type Perm Perm **Protected Phases** 8 2 6 4 8 2 **Permitted Phases** 4 6 6 Actuated Green, G (s) 7.8 7.8 7.8 7.3 7.3 7.3 7.3 7.3 8.5 8.5 Effective Green, g (s) 9.0 9.0 9.0 8.5 8.5 8.5 Actuated g/C Ratio 0.38 0.38 0.38 0.36 0.36 0.36 0.36 0.36 4.2 4.2 4.2 4.2 4.2 4.2 4.2 Clearance Time (s) 4.2 3.0 3.0 3.0 3.0 3.0 Vehicle Extension (s) 3.0 3.0 3.0 660 438 674 573 Lane Grp Cap (vph) 485 672 696 485 v/s Ratio Prot 0.14 0.09 0.03 c0.16 0.13 0.01 0.20 v/s Ratio Perm 0.11 0.24 0.03 0.09 0.20 0.32 0.37 v/c Ratio 0.28 0.42 5.5 5.2 4.8 4.9 5.2 Uniform Delay, d1 5.0 5.1 5.3 1.00 1.00 1.00 1.00 1.00 1.00 1.00 **Progression Factor** 1.00 0.2 0.5 0.2 0.0 0.1 Incremental Delay, d2 0.3 0.3 0.4 4.9 5.0 5.3 5.4 5.7 6.0 5.4 Delay (s) 5.3 Α Α Α Level of Service Α Α Α Α Α 5.4 5.7 5.7 5.3 Approach Delay (s) Α Α Α Α Approach LOS Intersection Summary **HCM** Level of Service Α **HCM Average Control Delay** 5.5 0.49 **HCM** Volume to Capacity ratio 6.0 Sum of lost time (s) 23.5 Actuated Cycle Length (s) 52.5% ICU Level of Service Α Intersection Capacity Utilization

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	-NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		*	ቀ ቕ	************	75	ተ ኈ	-
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		3.0			3.0		3.0	3.0		3.0	3.0	
Lane Util. Factor		1.00			1.00		1.00	0.95		1.00	0.95	
Frt		0.95			0.96		1.00	1.00		1.00	0.96	
Flt Protected		0.98			0.99		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1736			1769		1770	3527		1770	3397	
Flt Permitted		0.90			0.95		0.44	1.00		0.37	1.00	
Satd. Flow (perm)		1595			1688		814	3527		684	3397	
Volume (vph)	47	37	52	8	30	18	165	641	15	18	376	138
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	52	41	58	9	33	20	183	712	17	20	418	153
RTOR Reduction (vph)	0	50	0	0	17	0	0	1	0	0	21	0
Lane Group Flow (vph)	0	101	0	0	45	0	183	728	0	20	550	0
Turn Type	Perm			Perm		***************************************	Perm			Perm		
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		8.0			8.0		53.6	53.6		53.6	53.6	
Effective Green, g (s)		9.2			9.2		54.8	54.8		54.8	54.8	
Actuated g/C Ratio		0.13			0.13		0.78	0.78		0.78	0.78	
Clearance Time (s)		4.2			4.2		4.2	4.2		4.2	4.2	
Vehicle Extension (s)		3.0			3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)		210			222		637	2761		535	2659	
v/s Ratio Prot								0.21			0.17	
v/s Ratio Perm		c0.09			0.04		c0.22			0.03		
v/c Ratio		0.48			0.20		0.29	0.26		0.04	0.21	
Uniform Delay, d1		28.2			27.1		2.1	2.1		1.7	2.0	
Progression Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2		1.7			0.4		1.1	0.2		0.1	0.2	
Delay (s)		29.9			27.6		3.3	2.3		1.8	2.1	
Level of Service		С			С		Α	Α		Α	Α	
Approach Delay (s)		29.9			27.6			2.5			2.1	
Approach LOS		С			C			Α.			Α	
Intersection Summary												4 2 2
HCM Average Control D			5.7	Н	CM Lev	el of Se	ervice		Α			
HCM Volume to Capacit	-		0.35	_			4.3		0.0			
Actuated Cycle Length (70.0		um of k				6.0			
Intersection Capacity Ut	Ilization	•	48.3%	IC	CU Leve	or Ser	vice		Α			
Analysis Period (min)			15									
c Critical Lane Group												

Timing Plan: AM PEAK

Link: Allison Rd, Walterscheid Blvd to US 85

Variable	Value	Comments
Travel Time (s)	55	Travel Time okay For Coordination
CF1	32	•
Traffic / Storage Space	0.06	Storage Space is adequate
CF2	6	
Proportion of Traffic In Platoon	0.74	Traffic moderately platooned
Ap, platoon adjustment	-4	• •
Main Street Volume (vph)	447	Low Volumes, coordination is lower priority
Av, volume adjustment	-5	
Cycle Length	24.9	at Walterscheid Blvd
Cycle Length	70	at US 85
Combined Cycle Length	70	
Cycle Length Increase	10.1	
Ac, Cycle Adjustment	-5	
CF, Coordinatability Factor	18	Coordination definitely not recommended

Timing Plan: AM PEAK

Year 2019

Timing Plan: AM PEAK

10/15/2009 159: Allison Rd & Walterscheid Blvd Movement **EBL EBT** EBR WBL WBT **WBR NBL** NBT **NBR** SBL SBT **SBR** Lane Configurations 1 4 ħ 1900 1900 1900 1900 1900 1900 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 Total Lost time (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 1.00 1.00 Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00 0.94 0.98 0.85 Frt 1.00 0.99 1.00 1.00 1.00 1.00 Flt Protected 0.95 1.00 1.00 0.95 1.00 0.95 1.00 Satd. Flow (prot) 1770 1754 1847 1770 1826 1770 1863 1583 **Flt Permitted** 0.64 1.00 1.00 0.67 1.00 0.98 0.72 1.00 1826 1194 1863 Satd. Flow (perm) 1243 1754 1813 1334 1583 22 12 299 Volume (vph) 129 146 93 12 249 13 170 145 57 Peak-hour factor, PHF 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 189 161 24 13 63 332 Adi, Flow (vph) 143 162 103 13 277 14 0 212 0 0 9 0 0 RTOR Reduction (vph) 38 0 0 0 3 0 0 301 0 189 176 0 13 63 120 Lane Group Flow (vph) 143 227 Perm Perm Perm Turn Type Perm Perm 8 2 6 **Protected Phases** 4 2 6 8 6 Permitted Phases 4 7.4 7.4 7.4 7.4 7.9 7.9 7.9 7.4 Actuated Green, G (s) 8.6 8.6 8.6 8.6 8.6 9.1 9.1 9.1 Effective Green, a (s) 0.38 0.36 0.36 0.36 0.36 0.36 Actuated g/C Ratio 0.38 0.38 4.2 4.2 4.2 4.2 4.2 4.2 Clearance Time (s) 4.2 4.2 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 Vehicle Extension (s) 574 663 433 676 Lane Grp Cap (vph) 477 673 696 484 0.03 0.15 0.10 v/s Ratio Prot 0.01 0.21 c0.17 0.14 0.12 v/s Ratio Perm 0.27 0.03 0.09 0.21 0.39 0.30 0.34 0.43 v/c Ratio 5.0 5.2 5.6 5.3 4.9 5.4 5.1 5.2 Uniform Delay, d1 1.00 1.00 1.00 1.00 1.00 1.00 **Progression Factor** 1.00 1.00 0.2 0.0 0.1 0.5 0.2 0.4 0.3 0.4 Incremental Delay, d2 4.9 5.0 5.4 5.8 6.1 5.5 5.5 Delay (s) 5.4 Α Α Α Α Α Α Α Level of Service Α 5.8 5.3 5.8 5.5 Approach Delay (s) Α Α Α Approach LOS Α Intersection Summary Α **HCM** Level of Service **HCM** Average Control Delay 5.6 0.51 **HCM** Volume to Capacity ratio 6.0

HCM Average Control Delay

HCM Volume to Capacity ratio

Actuated Cycle Length (s)

Intersection Capacity Utilization

Analysis Period (min)

C Critical Lane Group

5.6

HCM Level of Service

A

HCM Level of Service

A

LCU Level of Service

A

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			€ }		T.	14		B	የ ጉ	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		3.0			3.0		3.0	3.0		3.0	3.0	
Lane Util. Factor		1.00			1.00		1.00	0.95		1.00	0.95	
Frt		0.95			0.96		1.00	1.00		1.00	0.96	
Fit Protected		0.98			0.99		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1736			1768		1770	3527		1770	3404	
Flt Permitted		0.89			0.94		0.41	1.00		0.34	1.00	
Satd. Flow (perm)		1579			1679		772	3527		626	3404	
Volume (vph)	50	39	55	9	32	20	167	708	17	20	415	141
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	56	43	61	10	36	22	186	787	19	22	461	157
RTOR Reduction (vph)	0	51	0	0	19	0	0	1	0	0	19	0
Lane Group Flow (vph)	Ö	109	Ö	Ō	49	0	186	805	0	22	599	0
Turn Type	Perm			Perm			Perm			Perm		
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		8.3			8.3		53.3	53.3		53.3	53.3	
Effective Green, g (s)		9.5			9.5		54.5	54.5		54.5	54.5	
Actuated g/C Ratio		0.14			0.14		0.78	0.78		0.78	0.78	
Clearance Time (s)		4.2			4.2		4.2	4.2		4.2	4.2	
Vehicle Extension (s)		3.0			3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)		214			228		601	2746		487	2650	
v/s Ratio Prot								0.23			0.18	
v/s Ratio Perm		c0.10			0.04		c0.24			0.04		
v/c Ratio		0.51			0.21		0.31	0.29		0.05	0.23	
Uniform Delay, d1		28.1			26.9		2.3	2.2		1.8	2.1	
Progression Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2		1.9			0.5		1.3	0.3		0.2	0.2	
Delay (s)		30.0			27.4		3.6	2.5		2.0	2.3	
Level of Service		С			С		Α	Α		Α	Α	
Approach Delay (s)		30.0			27.4			2.7			2.3	
Approach LOS		С			С			Α			Α	
Intersection Summary										V States		
HCM Average Control D	elay		5.8	F	ICM Le	vel of Se	ervice		· A			
HCM Volume to Capacit			0.37									
Actuated Cycle Length (s)		70.0			ost time	• •		6.0			
Intersection Capacity Ut			50.1%	10	CU Lev	el of Ser	vice		Α			
Analysis Period (min)			15									
c Critical Lane Group												

Timing Plan: AM PEAK

Timing Plan: AM PEAK 10/15/2009

Link: Allison Rd, Walterscheid Blvd to US 85

Variable	Value	Comments
Travel Time (s)	55	Travel Time okay For Coordination
CF1	32	•
Traffic / Storage Space	0.06	Storage Space is adequate
CF2	6	• .
Proportion of Traffic In Platoon	0.74	Traffic moderately platooned
Ap, platoon adjustment	-4	• •
Main Street Volume (vph)	464	Low Volumes, coordination is lower priority
Av, volume adjustment	-5	
Cycle Length	25.4	at Walterscheid Blvd
Cycle Length	70	at US 85
Combined Cycle Length	70	
Cycle Length Increase	9.6	
Ac, Cycle Adjustment	-4	
CF, Coordinatability Factor	19	Coordination definitely not recommended

Timing Plan: AM PEAK 10/15/2009

	۶		*	•	4	1	4	†	~	1	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL.	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	Þ			4		ħ	ĵ.		ħ	*	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.0			3.0		3.0	3.0		3.0	3.0	3.0
Lane Util. Factor	1.00	1.00			1.00		1.00	1.00		1.00	1.00	1.00
Frt	1.00	0.94			0.99		1.00	0.98		1.00	1.00	0.85
Flt Protected	0.95	1.00			1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1770	1754			1846		1770	1826		1770	1863	1583
Flt Permitted	0.65	1.00			0.98		0.71	1.00		0.63	1.00	1.00
Satd. Flow (perm)	1212	1754			1809		1325	1826		1172	1863	1583
Volume (vph)	136	155	98	13	255	14	178	160	24	13	63	310
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	151	172	109	14	283	16	198	178	27	14	70	344
RTOR Reduction (vph)	0	39	0	0	3	0	0	9	0	0	0	217
Lane Group Flow (vph)	151	242	. 0	0	310	0	198	196	0	14	70	127
Turn Type	Perm			Perm			Perm			Perm		Perm
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		6
Actuated Green, G (s)	8.1	8.1			8.1		7.7	7.7		7.7	7.7	7.7
Effective Green, g (s)	9.3	9.3			9.3		8.9	8.9		8.9	8.9	8.9
Actuated g/C Ratio	0.38	0.38			0.38		0.37	0.37		0.37	0.37	0.37
Clearance Time (s)	4.2	4.2			4.2		4.2	4.2		4.2	4.2	4.2
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	466	674			695		487	672		431	685	582
v/s Ratio Prot		0.16						0.11			0.04	
v/s Ratio Perm	0.12				c0.17		0.15			0.01		0.22
v/c Ratio	0.32	0.36			0.45		0.41	0.29		0.03	0.10	0.22
Uniform Delay, d1	5.2	5.3			5.5		5.7	5.4		4.9	5.0	5.3
Progression Factor	1.00	1.00			1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	0.4	0.3			0.5		0.6	0.2		0.0	0.1	0.2
Delay (s)	5.6	5.7			6.0		6.2	5.7		4.9	5.1	5.4
Level of Service	Α	Α			Α		Α	Α		Α	Α	Α
Approach Delay (s)		5.6			6.0			5.9			5.4	
Approach LOS		Α			Α			Α			Α	
Intersection Summary			7 (7) (2) (3) (3) (4) (4) (4) (4) (4) (4) (4) (4) (4) (4	4057								
HCM Average Control D			5.7	Н	CM Lev	el of Se	rvice		Α			
HCM Volume to Capacit	•		0.52	_								
Actuated Cycle Length (•	-	24.2						6.0			
Intersection Capacity Uti	lization		55.7%	IC	U Leve	of Ser	vice		В			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		*	ት ኩ		ሻ	作品	*************************************
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		3.0			3.0		3.0	3.0		3.0	3.0	
Lane Util. Factor		1.00			1.00		1.00	0.95		1.00	0.95	
Frt		0.95			0.96		1.00	1.00		1.00	0.96	
Flt Protected		0.98			0.99		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1736			1767		1770	3527		1770	3412	
Flt Permitted		0.89			0.94		0.39	1.00		0.30	1.00	
Satd. Flow (perm)		1571			1668		728	3527		567	3412	
Volume (vph)	53	41	59	10	34	22	169	782	19	22	458	144
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	59	46	66	11	38	24	188	869	21	24	509	160
RTOR Reduction (vph)	0	52	0	0	21	0	0	1	0	0	18	0
Lane Group Flow (vph)	0	119	0	0	52	0	188	889	0	24	651	0
Turn Type	Perm			Perm			Perm			Perm		
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		8.7			8.7		52.9	52.9		52.9	52.9	
Effective Green, g (s)		9.9			9.9		54.1	54.1		54.1	54.1	
Actuated g/C Ratio		0.14			0.14		0.77	0.77		0.77	0.77	
Clearance Time (s)		4.2			4.2		4.2	4.2		4.2	4.2	
Vehicle Extension (s)		3.0			3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)		222			236		563	2726		438	2637	
v/s Ratio Prot								0.25			0.20	
v/s Ratio Perm		c0.11			0.04		c0.26			0.04		
v/c Ratio		0.54			0.22		0.33	0.33		0.05	0.25	
Uniform Delay, d1		27.9			26.6		2.4	2.4		1.9	2.2	
Progression Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2		2.5			0.5		1.6	0.3		0.2	0.2	
Delay (s)		30.4			27.1		4.0	2.7		2.1	2.5	
Level of Service		С			С		Α	Α		Α	Α	
Approach Delay (s)		30.4			27.1			3.0			2.4	
Approach LOS		С			С			Α			Α	
Intersection Summary	Park Tale					V Late						
HCM Average Control D			6.0	Н	CM Lev	el of Se	rvice		Α			
HCM Volume to Capacit			0.40	_			<i>(</i>)		0.0			
Actuated Cycle Length (70.0			st time	• •		6.0			
Intersection Capacity Uti	lization	;	52.0%	IC	U Leve	l of Sen	vice		Α			
Analysis Period (min)			15									
c Critical Lane Group												

Timing Plan: AM PEAK

Link: Allison Rd, Walterscheid Blvd to US 85

Variable	Value	Comments
Travel Time (s)	55	Travel Time okay For Coordination
CF1	32	
Traffic / Storage Space	0.07	Storage Space is adequate
CF2	7	5 ,
Proportion of Traffic In Platoon	0.74	Traffic moderately platooned
Ap, platoon adjustment	-4	,,
Main Street Volume (vph)	484	Low Volumes, coordination is lower priority
Av, volume adjustment	-4	,
Cycle Length	26.1	at Walterscheid Blyd
Cycle Length	70	at US 85
Combined Cycle Length	70	
Cycle Length Increase	8.9	
Ac, Cycle Adjustment	-4	
CF, Coordinatability Factor	20	Coordination definitely not recommended

Timing Plan: AM PEAK

Year 2014

Timing Plan: PM PEAK

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	A			4		ħ	\$		76	4	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.0			3.0		3.0	3.0		3.0	3.0	3.0
Lane Util. Factor	1.00	1.00			1.00		1.00	1.00		1.00	1.00	1.00
Frt	1.00	0.95			0.98		1.00	0.96		1.00	1.00	0.85
Fit Protected	0.95	1.00			0.99		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1770	1777			1803		1770	1792		1770	1863	1583
Flt Permitted	0.80	1.00			0.89		0.61	1.00		0.66	1.00	1.00
Satd. Flow (perm)	1496	1777			1621		1139	1792		1237	1863	1583
Volume (vph)	149	186	83	35	109	29	46	98	33	22	213	69
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	166	207	92	39	121	32	51	109	37	24	237	77
RTOR Reduction (vph)	0	28	0	0	13	0	0	19	0	0	0	50
Lane Group Flow (vph)	166	271	0	0	179	0	51	127	0	24	237	27
Turn Type	Perm			Perm			Perm			Perm		Perm
Protected Phases		4			8		_	2			6	_
Permitted Phases	4			8			_ 2			6		6
Actuated Green, G (s)	8.0	8.0			8.0		7.2	7.2		7.2	7.2	7.2
Effective Green, g (s)	9.2	9.2			9.2		8.4	8.4		8.4	8.4	8.4
Actuated g/C Ratio	0.39	0.39			0.39		0.36	0.36		0.36	0.36	0.36
Clearance Time (s)	4.2	4.2			4.2		4.2	4.2		4.2	4.2	4.2
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	583	693			632		405	638		440	663	563
v/s Ratio Prot		c0.17						0.08		0.00	c0.13	0.05
v/s Ratio Perm	0.11				0.12		0.04	0.00		0.02	0.00	0.05
v/c Ratio	0.28	0.39			0.28		0.13	0.20		0.05	0.36	0.05
Uniform Delay, d1	4.9	5.2			4.9		5.1	5.3		5.0	5.6	5.0
Progression Factor	1.00	1.00			1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	0.3	0.4			0.2		0.1	0.2		0.1	0.3	0.0 5.0
Delay (s)	5.2	5.5			5.2		5.3	5.4		5.0	5.9	5.0 A
Level of Service	Α	Α			A		Α	A 5.4		Α	A 5.7	A
Approach Delay (s)		5.4			5.2 A			5.4 A			5.7 A	
Approach LOS		Α			A			^	eus are on their line in			
Intersection Summary			7 - 100 - 10				9.44.4	12.77.3				
HCM Average Control D			5.4	ŀ	ICM Lev	vel of Se	ervice		Α			
HCM Volume to Capacit			0.40	_					0.0			
Actuated Cycle Length (23.6						6.0			
Intersection Capacity Uti	lization		52.2%	10	JU Leve	ei ot Ser	vice		Α			
Analysis Period (min)			15									
c Critical Lane Group												

	•				4	4	•	+			 	
Movement	- EBL	EBT	▼ EBR	₩BL	NA/ID#	- Wob	T)	l Nese	<i>/</i>		*	****
Lane Configurations	LOL	 क	EDK	VVDL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	∢ĵ. 1900	1900	1900	ቶ ኬ 1900	1900	1900	^ 1900	1000
Lane Width	12	12	1300	1300	1300	12	1900	12	1900	1900	1900	1900
Total Lost time (s)	12	3.0	14	12	3.0	12	3.0	3.0	12	3.0	3.0	12
Lane Util. Factor		1.00			1.00		1.00	0.95		1.00	0.95	
Frt		0.95			0.93		1.00	1.00		1.00	0.95	
Flt Protected		0.98			0.99		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1734			1714		1770	3533		1770	3500	
Flt Permitted		0.87			0.94		0.27	1.00				
Satd. Flow (perm)		1535			1627					0.39	1.00	
	0.4		70	4.4			498	3533		720	3500	
Volume (vph)	94	44	78	11	15	29	62	586	7	45	804	64
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	104	49	87	12	17	32	69	651	8	50	893	71
RTOR Reduction (vph)	0	43	0	0	25	0	0	1	0	0	4	0
Lane Group Flow (vph)	0	197	0	0	36	0	69	658	0	50	960	0
Turn Type	Perm			Perm	_		Perm	_		Perm	_	
Protected Phases		4		_	8		_	2		_	6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		13.1			13.1		48.5	48.5		48.5	48.5	
Effective Green, g (s)		14.3			14.3		49.7	49.7		49.7	49.7	
Actuated g/C Ratio		0.20			0.20		0.71	0.71		0.71	0.71	
Clearance Time (s)		4.2			4.2		4.2	4.2		4.2	4.2	
Vehicle Extension (s)		3.0		.,	3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)		314			332		354	2508		511	2485	
v/s Ratio Prot								0.19			c0.28	
v/s Ratio Perm		c0.16			0.04		0.14			0.07		
v/c Ratio		0.63			0.11		0.19	0.26		0.10	0.39	
Uniform Delay, d1		25.4			22.7		3.4	3.6		3.2	4.1	
Progression Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2		3.9			0.1		1.2	0.3		0.4	0.5	
Delay (s)		29.3			22.8		4.6	3.9		3.5	4.5	
Level of Service		С			С		Α	Α		Α	Α	
Approach Delay (s)		29.3			22.8			3.9			4.5	
Approach LOS		С			С			Α			Α	
Intersection Summary												
HCM Average Control D	•		7.7	Н	CM Lev	el of Se	rvice		Α			
HCM Volume to Capacity			0.47	~								
Actuated Cycle Length (s			70.0			st time			6.0			
Intersection Capacity Uti	lization		56.7%	IC	U Leve	l of Ser	vice		В			
Analysis Period (min)			15									
c Critical Lane Group												

Timing Plan: PM PEAK

Timing Plan: PM PEAK 10/15/2009

Link: Allison Rd, Walterscheid Blvd to US 85

Variable	Value	Comments
Travel Time (s)	55	Travel Time okay For Coordination
CF1	32	•
Traffic / Storage Space	0.05	Storage Space is adequate
CF2	5	
Proportion of Traffic In Platoon	0.76	Traffic moderately platooned
Ap, platoon adjustment	-3	, ,
Main Street Volume (vph)	432	Low Volumes, coordination is lower priority
Av, volume adjustment	-5	•
Cycle Length	24.3	at Walterscheid Blvd
Cycle Length	70	at US 85
Combined Cycle Length	70	
Cycle Length Increase	10.7	
Ac, Cycle Adjustment	-5	
CF, Coordinatability Factor	19	Coordination definitely not recommended

EBL

1900

3.0

1.00

1.00

0.95

1770

0.78

1448

151

0.90

168

168

Perm

0

4

8.1

9.3

4.2

3.0

561

0.12

0.30

5.1

1.00

0.3

5.4

Α

0.39

EBT

1900

3.0

1.00

0.95

1.00

1777

1.00

1777

191

0.90

212

27

4

8.1

9.3

0.39

4.2

3.0

689

c0.17

0.41

5.3

1.00

0.4

5.7

5.6

Α

Α

279

ß

EBR WBL

1900

39

43

0

0

8

Perm

0.90

1900

85

94

0

0

0.90

WBT

1900

3.0

1.00

0.98

0.99

1800

0.89

1610

115

0.90

128

13

8

8.1

9.3

0.39

4.2

3.0

624

0.13

0.31

5.1

1.00

0.3

5.4

5.4

Α

Α

1.00

0.2

5.3

Α

1.00

0.2

5.5

5.4

Α

Α

194

4

Movement

Lane Configurations

Ideal Flow (vphpl)

Total Lost time (s)

Lane Util. Factor

Satd. Flow (prot)

Satd. Flow (perm)

Peak-hour factor, PHF

RTOR Reduction (vph)

Lane Group Flow (vph)

Fit Protected

Flt Permitted

Volume (vph)

Adj. Flow (vph)

Protected Phases

Permitted Phases

Actuated Green, G (s)

Effective Green, g (s)

Actuated g/C Ratio

Clearance Time (s)

Vehicle Extension (s)

Lane Grp Cap (vph)

v/s Ratio Prot

v/s Ratio Perm

Uniform Delay, d1

Level of Service

Approach LOS

Progression Factor

Approach Delay (s)

Incremental Delay, d2

v/c Ratio

Delay (s)

Turn Type

Frt

Timing Plan: PM PEAK 10/15/2009 WBR. NBL NBT **NBR** SBL SBT SBR ኘ 1900 1 1900 1900 1900 1900 1900 1900 3.0 3.0 3.0 3.0 3.0 1.00 1.00 1.00 1.00 1.00 1.00 0.96 1.00 1.00 0.85 0.95 1.00 0.95 1.00 1.00 1770 1793 1770 1863 1583 0.60 1.00 0.66 1.00 1.00 1114 1793 1221 1863 1583 32 49 108 36 24 235 71 0.90 0.90 0.90 0.90 0.90 0.90 0.90 36 54 120 40 261 27 79 0 0 19 0 0 0 50 0 54 141 27 0 261 29 Perm Perm Perm 2 6 2 6 6 7.5 7.5 7.5 7.5 7.5 8.7 8.7 8.7 8.7 8.7 0.36 0.36 0.36 0.36 0.36 4.2 4.2 4.2 4.2 4.2 3.0 3.0 3.0 3.0 3.0 404 650 443 675 574 0.09 c0.14 0.05 0.02 0.05 0.13 0.22 0.06 0.39 0.05 5.1 5.0 5.7 5.0 5.3

1.00

0.4

6.0

5.7

Α

Α

1.00

0.0

5.0

Α

1.00

0.1

5.0

Α

Intersection Summary			
HCM Average Control Delay	5.6	HCM Level of Service	Α
HCM Volume to Capacity ratio	0.42		ı
Actuated Cycle Length (s)	24.0	Sum of lost time (s)	6.0
Intersection Capacity Utilization	54.4%	ICU Level of Service	Α
Analysis Period (min)	15		
c Critical Lane Group			

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Movement	EBL	EBT	EBR	₩BL	WBT	WBR	NBL	I NBT	/ NBR	SBL	▼ SBT	SBR
Lane Configurations		4	LUIN	VVDL	4	AADIX	ivel ो		NDI	SDL ħ		ODR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	ት ጮ 1900	1900	1900	↑î → 1900	1900
Lane Width	12	12	12	1300	1300	1300	12	12	1900	12	1900	12
Total Lost time (s)	12	3.0	12.	12	3.0	12	3.0	3.0	12	3.0	3.0	12
Lane Util. Factor		1.00			1.00		1.00	0.95		1.00	0.95	
Frt		0.95			0.93		1.00	1.00		1.00	0.99	
Fit Protected		0.98			0.99		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1733			1713		1770	3533		1770	3501	
Flt Permitted		0.86			0.94		0.23	1.00		0.36	1.00	
Satd. Flow (perm)		1528			1624		436	3533		662	3501	
Volume (vph)	97	45	82	12	17	32	67	647	8	50	888	69
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	108	50	91	13	19	36	74	719	9	56	987	77
RTOR Reduction (vph)	0	43	0	. 0	28	0	0	1	Ō	0	4	0
Lane Group Flow (vph)	Ō	206	Ö	Ō	40	Ö	74	727	Ō	56	1060	Ō
Turn Type	Perm			Perm	****		Perm			Perm		
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		13.5			13.5		48.1	48.1		48.1	48.1	
Effective Green, g (s)		14.7			14.7		49.3	49.3		49.3	49.3	
Actuated g/C Ratio		0.21			0.21		0.70	0.70		0.70	0.70	
Clearance Time (s)		4.2			4.2		4.2	4.2		4.2	4.2	
Vehicle Extension (s)		3.0			3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)		321			341		307	2488		466	2466	
v/s Ratio Prot								0.21			c0.30	
v/s Ratio Perm		c0.16			0.04		0.17			0.08		
v/c Ratio		0.64			0.12		0.24	0.29		0.12	0.43	
Uniform Delay, d1		25.2			22.4		3.7	3.9		3.3	4.4	
Progression Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2		4.3			0.2		1.9	0.3		0.5	0.5	
Delay (s)		29.6			22.5		5.5	4.2		3.9	4.9	
Level of Service		С			C		Α	Α		Α	Α	
Approach Delay (s)		29.6			22.5			4.3			4.9	
Approach LOS		С	25 NO. 10 TH NO. 25	AND THE STREET STREET	С	rr (vto 600 \$100 \$100 \$1)	e successor son	A	men dayarah saer ekka	(1) (1) (1) (1) (1) (1) (1) (1) (1) (1)	A	
Intersection Summary			7.0	11	CMIO	/el of Se	nuina		A		de joint se	
HCM Average Control D			7.9	н	CIVI LE	/ei 01 06	ii vice		A			
HCM Volume to Capacit			0.51 70.0	c	um of k	ost time	(e)		6.0			
Actuated Cycle Length (70.0 59.9%			el of Ser			0.0 B			
Intersection Capacity Uti Analysis Period (min)	mzauon	;	15	10	JO LEVE	, oi oei	*10C					
c Critical Lane Group			10									
c Critical Lane Group												

Timing Plan: PM PEAK

Link: Allison Rd, Walterscheid Blvd to US 85

Variable	Value	Comments
Travel Time (s)	55	Travel Time okay For Coordination
CF1	32	
Traffic / Storage Space	0.05	Storage Space is adequate
CF2	5	
Proportion of Traffic In Platoon	0.76	Traffic moderately platooned
Ap, platoon adjustment	-3	
Main Street Volume (vph)	456	Low Volumes, coordination is lower priority
Av, volume adjustment	- 5	
Cycle Length	25.1	at Walterscheid Blvd
Cycle Length	70	at US 85
Combined Cycle Length	70	
Cycle Length Increase	9.9	
Ac, Cycle Adjustment	-4	
CF, Coordinatability Factor	20	Coordination definitely not recommended

Timing Plan: PM PEAK

Timing Plan: PM PEAK

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	ĵ»			Ą.		ħ	Þ		ħ	4	ř
ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.0			3.0		3.0	3.0		3.0	3.0	3.0
Lane Util. Factor	1.00	1.00			1.00		1.00	1.00		1.00	1.00	1.00
Frt	1.00	0.95			0.98		1.00	0.96		1.00	1.00	0.85
Flt Protected	0.95	1.00			0.99		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1770	1777			1799		1770	1793		1770	1863	1583
Flt Permitted	0.75	1.00			0.88		0.58	1.00		0.65	1.00	1.00
Satd. Flow (perm)	1402	1777			1595		1087	1793		1204	1863	<u> 1583</u>
Volume (vph)	153	197	87	43	122	35	52	119	40	26	259	74
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	170	219	97	48	136	39	58	132	44	29	288	82
RTOR Reduction (vph)	0	26	0	0	13	0	0	20	0	0	0	52
Lane Group Flow (vph)	170	290	0	0	210	0	58	156	0	29	288	30
Turn Type	Perm			Perm			Perm			Perm		Perm
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		6
Actuated Green, G (s)	8.2	8.2			8.2		7.7	7.7		7.7	7.7	7.7
Effective Green, g (s)	9.4	9.4			9.4		8.9	8.9		8.9	8.9	8.9
Actuated g/C Ratio	0.39	0.39			0.39		0.37	0.37		0.37	0.37	0.37
Clearance Time (s)	4.2	4.2			4.2		4.2	4.2		4.2	4.2	4.2
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0	,	3.0	3.0	3.0
Lane Grp Cap (vph)	542	687			617		398	657		441	682	580
v/s Ratio Prot		c0.18						0.10			c0.15	
v/s Ratio Perm	0.12				0.14		0.05			0.02		0.05
v/c Ratio	0.31	0.42			0.34		0.15	0.24		0.07	0.42	0.05
Uniform Delay, d1	5.2	5.5			5.3		5.2	5.3		5.0	5.8	5.0
Progression Factor	1.00	1.00			1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	0.3	0.4			0.3		0.2	0.2		0.1	0.4	0.0
Delay (s)	5.5	5.9			5.6		5.3	5.5		5.1	6.2	5.0
Level of Service	Α	Α			Α		Α	Ā		Α	. A	Α
Approach Delay (s)		5.8			5.6			5.5			5.9	
Approach LOS		Α			Α			Α			Α	
Intersection Summary												
HCM Average Control D			5.7	F	ICM Lev	vel of Se	ervice		Α			
HCM Volume to Capacit			0.44	=	. <u>-</u> -				•			
Actuated Cycle Length (24.3			ost time			6.0			
Intersection Capacity Ut	ilization		56.9%	10	JU Leve	el of Ser	vice		В			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4].		*	ተ Ъ		N.	ተ ጮ	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	12	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		3.0			3.0		3.0	3.0		3.0	3.0	
Lane Util. Factor		1.00			1.00		1.00	0.95		1.00	0.95	
Frt		0.95			0.93		1.00	1.00		1.00	0.99	
Flt Protected		0.98			0.99		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1732			1714		1770	3533		1770	3502	
Fit Permitted		0.86			0.94		0.20	1.00		0.32	1.00	
Satd. Flow (perm)		1521			1626		373	3533		600	3502	
Volume (vph)	101	46	86	13	19	35	72	714	9	55	980	74
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	112	51	96	14	21	39	80	793	10	61	1089	82
RTOR Reduction (vph)	0	33	0	0	30	0	0	1	0	0	4	0
Lane Group Flow (vph)	0	226	0	0	44	0	80	802	0	61	1167	0
Turn Type	Perm	,		Perm			Perm		· · · · · · · · · · · · · · · · · · ·	Perm		
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		14.4			14.4		47.2	47.2		47.2	47.2	
Effective Green, g (s)		15.6			15. 6		48.4	48.4		48.4	48.4	
Actuated g/C Ratio		0.22			0.22		0.69	0.69		0.69	0.69	
Clearance Time (s)		4.2			4.2		4.2	4.2		4.2	4.2	
Vehicle Extension (s)		3.0			3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)		339			362		258	2443		415	2421	
v/s Ratio Prot								0.23			c0.33	
v/s Ratio Perm		c0.17			0.05		0.21			0.10		
v/c Ratio		0.67			0.12		0.31	0.33		0.15	0.48	
Uniform Delay, d1		24.8			21.7		4.2	4.3		3.7	5.0	
Progression Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2		4.9			0.2		3.1	0.4		0.7	0.7	
Delay (s)		29.7			21.9		7.3	4.7		4.5	5.7	
Level of Service		С			С		Α	Α		Α	Α	
Approach Delay (s)		29.7			21.9			4.9			5.6	
Approach LOS		C			С			Α			Α	
Intersection Summary												
HCM Average Control D			8.4	Н	CM Lev	el of Se	ervice		Α			
HCM Volume to Capacit			0.55	_			, ,					
Actuated Cycle Length (70.0			ost time			6.0			
Intersection Capacity Uti Analysis Period (min)	lization	(63.4% 15	IC	CU Leve	el of Ser	vice		В			
c Critical Lane Group												

Timing Plan: PM PEAK 10/15/2009

Link: Allison Rd, Walterscheid Blvd to US 85

Variable	Value	Comments
Travel Time (s)	55	Travel Time okay For Coordination
CF1	32	
Traffic / Storage Space	0.05	Storage Space is adequate
CF2	5	
Proportion of Traffic In Platoon	0.75	Traffic moderately platooned
Ap, platoon adjustment	-4	
Main Street Volume (vph)	482	Low Volumes, coordination is lower priority
Av, volume adjustment	-4	
Cycle Length	25.9	at Walterscheid Blvd
Cycle Length	70	at US 85
Combined Cycle Length	70	
Cycle Length Increase	9.1	
Ac, Cycle Adjustment	-4	
CF, Coordinatability Factor	20	Coordination definitely not recommended

Timing Plan: PM PEAK